## NOTICES OF PROPOSED RULEMAKING

Unless exempted by A.R.S. § 41-1005, each agency shall begin the rulemaking process by first submitting to the Secretary of State's Office a Notice of Rulemaking Docket Opening followed by a Notice of Proposed Rulemaking that contains the preamble and the full text of the rules. The Secretary of State's Office publishes each Notice in the next available issue of the Register according to the schedule of deadlines for Register publication. Under the Administrative Procedure Act (A.R.S. § 41-1001 et seq.), an agency must allow at least 30 days to elapse after the publication of the Notice of Proposed Rulemaking in the Register before beginning any proceedings for making, amending, or repealing any rule. (A.R.S. §§ 41-1013 and 41-1022)

## NOTICE OF PROPOSED RULEMAKING

## TITLE 4. PROFESSIONS AND OCCUPATIONS

## CHAPTER 48. ARIZONA UNIFORM PLUMBING CODE COMMISSION

## PREAMBLE

1. | Sections Affected |
| :---: |
| R4-48-101 |
| R4-48-102 |
| R4-48-103 |
| R4-48-104 |
| R4-48-105 |
| Table A |
| R4-48-106 |
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| R4-48-110 |
| R4-48-111 |
| R4-48-112 |
| R4-48-113 |
| R4-48-114 |
| Table A |
| R4-48-121 |
| R4-48-122 |
| Illustration A |
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| Table A |
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| Appendix A |
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| R4-48-143 |
| R4-48-147 |
| Table A |
| Table B |
| Table C |
| Tser |

## Rulemaking Action

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2. The specific authority for the rulemaking, including both the authorizing statute (general) and the statutes the rules are implementing (specific):

Authorizing statute: A.R.S. § 41-619
Implementing statute: A.R.S. § 41-619
3. A list of all previous notices appearing in the Register addressing the proposed rules:

Notice of Rulemaking Docket Opening: 10 A.A.R. 3404, August 27, 2004
4. The name and address of agency personnel with whom persons may communicate regarding the rulemaking: Name: Brian Townsend, Chairperson
Address: Arizona Uniform Plumbing Code Commission c/o Arizona Registrar of Contractors
800 W. Washington, 6th Floor
Phoenix, AZ 85007
Telephone: (602) 542-1525, Ext. 7652
Fax: (602) 542-7852
E-mail sharon.kowalski@roc1.rc.state.az.us
5. An explanation of the rules, including the agency's reasons for initiating the rules:

Over the past 3 years, the Arizona Uniform Plumbing Code Commission has adopted and amended the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) 1994 Uniform Plumbing Code (U.P.C.), including its appendices and installation standards, as the state plumbing code.

Advancing to the latest edition of every construction Code is standard practices in most jurisdictions that utilize model Codes. Because it took a number of years to adopt the 1994 UPC as the State of Arizona Plumbing Code, consideration of the 1997 edition of the UPC has passed. In reality the 1994 UPC was only recently fully implemented as the State of Arizona Plumbing Code. Now that the industry has a finalized State of Arizona Plumbing Code in place it would be appropriate for the Uniform Plumbing Code Commission to review and adopt a newer published version (the 2000 UPC) of this basic Code document, including approved amendments.

On August 22, 2002, the Commission met to review 2 code change submittals. The Commission agreed to the following submittal: "The 1994 Uniform Plumbing Code [as published by the International Association of Plumbing \& Mechanical Officials] was fully adopted by the State of Arizona with final rulemaking 5 A.A.R 802, effective February 24, 1999 (Supp.99-1). The 2000 edition of the Uniform Plumbing Code is in current publication and available to all interested parties. It is hereby recommended that the 2000 Uniform Plumbing Code be reviewed for adoption, and that all existing State of Arizona Plumbing Code amendments by reviewed for applicability to this newer edition (2000) of the Uniform Plumbing Code. This proposal would exclude adoption of Chapter 1 (Administration) and any Fee Schedule(s) related to Chapter 1." This proposed rulemaking is in preparation to submit those amendments for final rulemaking.
6. A reference to any study relevant to the rules that the agency reviewed and either proposes to rely on in its evaluation of or justification for the rules, where the public may obtain or review each study, all data underlying each study, and any analysis of each study and other supporting material: None
7. A showing of good cause why the rules are necessary to promote a statewide interest if the rules will diminish a previous grant of authority of a political subdivision of this state:

The 2000 Uniform Plumbing Code is comprised of essentially the same material as that contained in Arizona's existing (1994) State Plumbing Code. Adoption of the 2000 version of the UPC would create virtually no disruption, and would require minimal training for those who use the Plumbing Code. Whether the UPC serves its users in the industry as a design manual or as an installation guideline, this proposal to transition from the existing State of Arizona Code ( 1994 UPC) to the newly adopted 2000 UPC would represent the least disruptive upgrade possible.

Plumbing Codes other than the Uniform Plumbing Code may be based upon different philosophical perspectives, and in some cases are founded upon technical premises that are inconsistent with those which serve as the basis for the Uniform Plumbing Code. The Uniform Plumbing Code (originally known as the Western Plumbing Officials Plumbing Code) has been Arizona's plumbing code since the 1940 's. The State of Arizona Plumbing Contractor's License examinations have been (and are to this day) based upon what is now called the Uniform Plumbing Code. The principle alternative to the UPC is an amalgamated Code that was compiled using elements of three separate Plumbing Codes that are used on the East Coast, in the Mid-West, and in the Southeastern part of the United States. It is only reasonable that Arizona would be better served by remaining with this version of a familiar Code (the Uniform Plumbing Code), a document that has served the health and safety of Arizona's citizens well for more than 70 years.
8. The summary of the economic, small business, and consumer impact:

The benefit gained by adopting the current version of the Uniform Plumbing Code is the limited cost impact. By adopting the 2000 edition of a previously reviewed and adopted Plumbing Code the costs to the industry would be minimal because users of the State Plumbing Code are familiar with the basic document. Costs related to re-training would be minimal because designers and installers are already familiar with the language and intent of the Uniform Plumbing Code. Upgrading to this version of the Uniform Plumbing Code would constitute little more than becoming familiar with the latest amendments.

- Impacted Parties - No appreciable impact to any aspect of the construction industry.


## Notices of Proposed Rulemaking

- Probable Impacts on Employment - None. The labor requirements would remain relatively constant. Because the cost of a plumbing system is an insignificant proportion of the cost of construction, minor variables in Code requirements would have an even less significant impact on building costs. Minor variables in construction costs have little or no effect on the decision to build or not, thus employment would be virtually unaffected.
- Probable Impacts on Small Business - None (See Above)
- Probable Impacts on State Revenues - None (Same basic Code)
- Probable Impacts on Political Subdivisions of the State - None (Same basic Code)
- Less Intrusive or less costly alternatives to proposed changes - See second dot point from the top. Less intrusive or less costly alternatives may result in less of a plumbing system. Commentary: Early plumbing in the City of Chicago was designed using S-trapped single pipe plumbing with no vents other than a house vent at the lower end of the drainage system. It was relatively cheap with unsatisfactory performance, largely because it was less plumbing than it should have been. However, this early day (late1800's) plumbing had just barely moved indoors and was lacking in engineering and design sophistication. Minimal plumbing was shown to be no more of a bargain than a cheap parachute that opens most of the time. Less quality-less cost may be appealing to the homebuilder, but its usually no long term bargain for the home owner.
- Summary of the Estimated Economic Impacts - Minimal if any.

9. The name and address of agency personnel with whom persons may communicate regarding the accuracy of the economic, small business, and consumer impact statement:

Name: Brian Townsend, Chairperson
Address: Arizona Uniform Plumbing Code Commission
c/o Arizona Registrar of Contractors
800 W. Washington, 6th Floor
Phoenix, AZ 85007
Telephone: (602) 542-1525, Ext. 7652
Fax: (602) 542-7852
E-mail sharon.kowalski@roc1.rc.state.az.us
10. The time, place, and nature of the proceedings for the making, amendment, or repeal of the rules, or if no proceeding is scheduled, where, when, and how persons may request an oral proceeding on the proposed rules:

The Arizona Uniform Plumbing Code Commission meets at least once each year and at additional times on the call of the chairperson or a majority of its members. Oral comments will be accepted at these meetings until the close of record. Written comments will be accepted until the close of record at the location listed in question \#4 from 8:30 a.m. to 5:00 p.m., Monday through Friday.
11. Any other matters prescribed by statute that are applicable to the specific agency or to any specific rule or class of rules:

None

## 12. Incorporations by reference and their location in the rules:

Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Hot- and Cold- Water Distribution Systems, ASTM D 2846/D 2846M-99:

R4-48-103. Chapter 3, General Provisions, page 12
Solvent Cements for Chlorinated Poly (Vinyl Chloride) (CPVC) Plastic Pipe and Fittings, ASTM F493-97:
R4-48-103. Chapter 3, General Provisions, page 12
Air Admittance Valves for Plumbing Drainage Systems, ASSE Standard 1051:
R4-48-109. Chapter 9, Vents, page 22
NFPA 99 Standard for Health Care Facilities, 2002 edition:
R4-48-113. Chapter 13, Health Care Facilities and Medical Gas and Vacuum Systems, page 26
NFPA 99C: Standard on Gas and Vacuum Systems, 2002 edition:
R4-48-113. Chapter 13, Health Care Facilities and Medical Gas and Vacuum Systems, page 26
Crosslinked Polyethylene/Aluminum/Crosslinked Polyethylene (PEX-AL-PEX) Pressure Pipe, ASTM F1281-02e1:
R4-48-114. Chapter 14, Mandatory Referenced Standards, page 26
Polyethylene/Aluminum/Polyethylene (PE-AL-PE) Composite Pressure Pipe, ASTM F 1282-02e1:
R4-48-114. Chapter 14, Mandatory Referenced Standards, page 26
Metal Insert Fittings for Polyethylene/Aluminum/Polyethylene and Crosslinked Polyethylene/Aluminum/ Crosslinked Polyethylene Composite Pressure Pipe, ASTM F1974-01e1:

R4-48-114. Chapter 14, Mandatory Referenced Standards, page 26
Standard Practice for Surface Site Characterization for On-Site Septic Systems, ASTM D-5879-95el:
R4-48-125. Appendix G, Graywater Systems for Single Family Dwellings, page 35
Standard Practice for Subsurface Site Characterization of Test Pits for On-Site Septic Systems ASTM D-5921-96:
R4-48-125. Appendix G, Graywater Systems for Single Family Dwellings, page 35
Standard Practice for Soil Investigation and Sampling by Auger Borings ASTM D-1452-80 (Reapproved 1995):
R4-48-125. Appendix G, Graywater Systems for Single Family Dwellings, page 35
Standard Specification for Precast Concrete Septic Tanks ASTM C1227-00:
R4-48-127. Appendix K, Private Sewage Disposal Systems page 39
Material and Property standard for Prefabricated Septic Tanks IAPMO PS1-93:
R4-48-127. Appendix K, Private Sewage Disposal Systems page 39
Building Code Requirements for Structural Concrete ACI 318-99:
R4-48-127. Appendix K, Private Sewage Disposal Systems page 39
Commentary ACI 318R-99:
R4-48-127. Appendix K, Private Sewage Disposal Systems page 39
Environmental Engineering Concrete Structures ACI 350R-89:
R4-48-127. Appendix K, Private Sewage Disposal Systems page 39

## 13. The full text of the rules follows:

## TITLE 4. PROFESSIONS AND OCCUPATIONS

## CHAPTER 48. ARIZONA UNIFORM PLUMBING CODE COMMISSION

## ARTICLE 1. ARIZONA UNIFORM PLUMBING CODE

Section
R4-48-101. Chapter 2, Definitions
R4-48-102. Incorporation of the Uniform Plumbing Code by Reference
R4-48-103. Chapter 3, General Provisions Regulations
R4-48-104. Chapter 4, Plumbing Fixtures and Fixture Fittings
R4-48-105. Chapter 5, Water Heaters
Table A. Size of Combustion Air Openings or Ducts for Gas- or Liquid- Burning Water Heaters
R4-48-106. Chapter 6, Water Supply and Distribution
R4-48-107. Chapter 7, Sanitary Drainage
R4-48-108. Chapter 8, Indirect and Special Wastes
R4-48-109. Chapter 9, Vents
R4-48-110. Chapter 10, Traps and Interceptors
R4-48-111. Chapter 11, Storm Drainage
R4-48-112. Chapter 12, Fuel Piping
R4-48-113. Chapter 13, Speeial Piping and Storage Systems Health Care Facilities and Medical Gas and Vacuum Systems
R4-48-114. Chapter 14, Mandatory Referenced Standards
Table A. Plumbing Material Standards Repealed
R4-48-121. Appendix C, Minimum Plumbing Facilities Sizing of Category I Venting Systems
R4-48-122. Appendix D, Rainwater Systems Reserved
Illustration A. Repealed
R4-48-123. Appendix E, Manufactured or/ Mobile Home Parks and Recreational Vehicle Parks
R4-48-124. Appendix F, Medical Gas Systems Reserved
R4-48-125. Appendix G, Graywater Systems for Single Family Dwellings
Table A. Irrigation Disposal Fields
Table B. Location of Graywater System and Setback Requirements
Table C. Effluent Application Loading Rates to Soil for Graywater Systems
R4-48-126. Appendix H, Procedures for Sizing Commercial Kitchen Grease Interceptors Recommended Procedures for Design, Construction and Installation of Commercial Kitchen Grease Interceptors
R4-48-127. Appendix IK , Private Sewage Disposal Systems
Appendix A. Modified Appendix I Repealed
R4-48-128. Appendix J, Reclaimed Water Systems

## Notices of Proposed Rulemaking

R4-48-138. Installation Standard for Poly (Vinyl Chloride) (PVC) Natural Gas Yard Piping Reserved

R4-48-143. Installation Standard for Lower Low Pressure Air Test for Building Sewers
R4-48-147. Installation Standard for PEX-AL-PEX and PE-AL-PE Trenchless Polyethylene (PE) Pipe for Sewer Laterals
Table A. Repealed
Table B. Repealed
Table C. Repealed

## ARTICLE 1. ARIZONA UNIFORM PLUMBING CODE

## R4-48-101. Chapter 2. Definitions

A. The following definition applies in this Article: "Person" has the meaning set forth in A.R.S. § 1-215.
B. The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 202.0 203.0 Add a definition for "Air Admittance Valve" which reads:

Air Admittance Valve means a one-way valve designed to allow air to enter the plumbing drainage system when negative pressure develops in the piping system.
2. Sec. 202.0. 211.0 Modify the definition of "Insanitary" by replacing numbered paragraphs (2) and (3) with the following:
(2) Any opening in a drainage system, except where lawful, which is not provided with an approved liquid sealed trap.
(3) Any plumbing fixture or other waste discharging receptacle or device, which is not supplied with water sufficient to flush it and maintain it in a clean condition except those specifically designed to function without water.
3. Sec. 202.0. Add a definition for "PEX" which reads:
"PEX means Cross-linked Polyethylene."
4.3. Sec. 202.0 218.0 Add the following definition:

PE-AL-PE - Polyethylene-Aluminum-Polyethylene
5.4. Sec. 202.0 218.0 Add the following definition:

PEX-AL-PEX - Crosslinked Polyethylene-Aluminum-Crosslinked Polyethylene

## R4-48-102. Incorporation of the Uniform Plumbing Code by Reference

A. All persons shall comply with the following:

1. All persons shall comply with the The International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code (1994 2000 Edition) which is incorporated by reference, including appendices A through J A, B, C, E, G, H, J and K, and installation standards, as the state plumbing code. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O., z0001 Walntt Drive South, Walntt, CA, 91789-2825 5001 E. Philadelphia St., Ontario, CA 91761 and are on file with Arizona Uniform Plumbing Code Commission. And the Office of the Secretary of State.
2. The National Fire Protection Association (NFPA) 99 Standard for Health Care Facilities (2002 Edition) which is incorporated by reference as part of the state plumbing code. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from NFPA, 11 Tracy Drive, Avon, MA, 02322 and are on file with the Arizona Uniform Plumbing Code Commission.
3. NFPA 99C Standard on Gas and Vacuum Systems (2002 Edition), which is incorporated by reference as part of the state plumbing code. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from NFPA, 11 Tracy Drive, Avon, MA, 02322 and are on file with the Arizona Uniform Plumbing Code Commission.
B. The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
4. Delete pages: one through 14 , Chapter 1 after making all modifications in this Article.
5. Replace Chapter 13 and incorporate by reference the National Fire Protection Association (NFPA) 99 Standard for Health Care Facilities (2002 Edition) and NFPA 99C Standard on Gas and Vacuum Systems (2002 Edition).
6. Delete all references to Chapter 15 and Appendix D from R4-48-111, Chapter 11 Storm Drainage.
7. Delete Chapter 15, Firestop Protection for DWV and Stormwater Applications.

## R4-48-103. Chapter 3, General Provisions Regulations

A. The rules of this Article are adopted under A.R.S. § 41-619, which provides for the adoption statewide of the Uniform Plumbing Code ("Code"). The rules do not specify the procedures by which the Code will be enforced, which may be governed by other provisions of state law. The rules do not supersede, restrict, or negate the authority of any state agency, municipality, or county to administer, delegate, or enforce laws, statutes, rules, or ordinances within its respective jurisdiction.
B. The Uniform Plumbing Code incorporated in R4-48-102(A), is modified as follows:

1. Move Sec. 101.4.1.3 to a new Sec. 301.1.5.
2. Move Sec. 101.5.2 to a new Sec. 301.1.6.
3. Move Sec. 101.5.4 to a new Sec. 301.1.7.
4. Move Sec. 101.5.5 to a new Sec. 301.1.8.
5. Move Sec. 103.5 to a new Sec. 318.0 320.0.
6. Move Sec. 103.5.1 to a new Sec. 318.1 320.1.
7. Move Sec. 103.5.1.2 to a new Sec. 318.2 320.2.
8. Move Sec. 103.5.1.3 to a new Sec. 318.3 320.3.
9. Move Sec. 103.5.1.4 to a new Sec. 318.4 320.4.
10. Move Sec. 103.5.5 to a new Sec. 318.5 320.5.
11. Move Sec. 103.5.5.1 to a new Sec. 318.6 320.6.
12. Move Sec. 103.5.5.2 to a new Sec. 318.7 320.7.
13. Move Sec. 103.5.3 to a new Sec. 319.0 321.0.
14. Move Sec. 103.5.3.1 to a new Sec. 319.1.
15.14. Move Sec. 103.5.3.2 to a new Sec. 319.2 321.1.
16.15. Move Sec. 103.5.3.4 to a new Sec. 319.3 321.2.
17.16. Move Sec. 103.5.3.5 to a new Sec. 319.4 321.3.
18.17. Move Sec. 103.5.3.7 to a new Sec. 319.5 321.4.
19.18. Move Sec. 103.5.6.1 to a new Sec. 319.6 321.5 .
20.19. Move Sec. 103.5.6.2 to a new Sec. 319.7 321.6.
21.20. Move Sec. 103.5.6.3 to a new Sec. 319.8 321.7.
15. Move See. 712.4 to a new See. 320.0.
16. Move Sec. 712.4.1 to a new Sec. 320.1.
17. Move Sec. 712.4.2 to a new Sec. 320.2.
18. Move Sec. 712.4.3 to a new Sec. 320.3.
19. Move Sec. 712.4.4 to a new Sec. 320.4.
27.21. Move Sec. 103.6 to a new Sec. 321.0 322.0.
28.22. Move Sec. 103.6.1 to a new Sec. 321.4 322.1.
29.23. Move Sec. 103.6.2 to a new Sec. 321.2 322.2.
30.24. Move Sec. 103.6.3 to a new Sec. 321.3 322.3.
31.25. Add three new row to Table 3-4 3-2. Refer to Table A - Materials and Types of Joints (Horizontal and Vertical).
32.26. Sec. 316.1.5 316.1.6 is modified to read:

Solvent Cement Plastic Pipe Joints. Plastic pipe and fittings designed to be joined by solvent cementing shall comply with appropriate IAPMO Installation Standards. ABS pipe and fittings shall be cleaned and then joined with listed solvent cement(s). CPVC pipe and fittings shall be cleaned and joined with listed primer(s) and solvent cement(s). Exception: Listed solvent cements that do not require the use of a primer shall be permitted for use with CPVC pipe and fittings $1 / 2$ " through 2 " in diameter, manufactured in accordance with ASTM D 2846/D 2846M-99, which is incorporated by reference and published by American Society for Testing and Materials, 100 Barr Harbor Drive, West Conshohocken, PA 19428-2959. This incorporation by reference does not include any later amendments or editions. PVC pipe and fittings shall be cleaned and joined with listed primer(s) and solvent cement(s) conforming to ASTM F 493-97, which is incorporated by reference and published by American Society for Testing and Materials, 100 Barr Harbor Drive, West Conshohocken, PA 19428-2959. This incorporation by reference does not include any later amendments or editions.

## R4-48-104. Chapter 4, Plumbing Fixtures and Fixture Fittings

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 402.1 is modified to read: The maximum flow rates and quantities for all plumbing fixtures and fixture fittings shall be consistent with A.R.S. Title 45, Chapter 1, Article 12.
2. Sec. 402.2 is deleted.
3. Add a third paragraph in Table 4-1 to read: Those jurisdictions that have not adopted a building code which stipulates minimum plumbing facilities shall utilize Table 4-1 of the 2000 Uniform Plumbing Code when establishing plumbing facility requirements.

## R4-48-105. Chapter 5, Water Heaters

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 501.0 is modified to read: "General. The regulations of this Chapter shall govern the construction, location, and installation of all fuel-burning and other water heaters heating potable water, together with all chimneys, vents, and their connectors. All design, construction, and workmanship shall be in conformity with accepted engineering practices and shall be of such character as to secure the results sought to be obtained by this Code. No water heater shall be hereinafter installed which does not comply in all respects with the type and model of each size thereof approved by the Administrative Authority. A list of generally accepted gas equipment standards is included in Table 14-1. A

## Notices of Proposed Rulemaking

water heater (boiler) which exceeds any of the following limitations shall not be placed in service until the vessel is separately inspected pursuant to A.R.S. Title 23, Chapter 2, Article 11.
(a) 120 -gallon $(454.2 \mathrm{~L})$ nominal water capacity.
(b) $160 \mathrm{PSI}(1,103.2 \mathrm{kPa})$ operating pressure.
(c) $210 \times \mathrm{F}(98.9 \times \mathrm{C})$ operating temperature.
(d) $200,000 \mathrm{btu} / \mathrm{h}(58,620 \mathrm{~W})$ heat input.

The minimum capacity for water heaters shall be in accordance with the first hour rating listed in Table 5-1. All design, construction, and workmanship shall be in conformity with accepted engineering practices, manufacturer's installation instructions, and applicable standards and shall be of such character as to secure the results sought to be obtained by this Code.
2. Sec. 507.2 is modified to read: In new construction, all enclosed buildings shall be provided with combustion air obtained from outside. In existing buildings of ordinary tightness insofar as infiltration is concerned, all or a portion of the combustion air for fuel-burning water heaters may be obtained from infiltration if the enclosure volume equals at least 50 cubic feet per $1000 \mathrm{btu} / \mathrm{h}(4.831 \mathrm{~L} / \mathrm{W})$ input of the water heater. Existing buildings of unusually tight construction shall also be provided with outside combustion air. No change in combustion air is required when an existing fuel-burning water heater is replaced with a new water heater having the same or smaller btu/h input capacity. When an existing fuel-burning water heater is replaced by a higher-capacity water heater, or when additional fuel burning water heaters are installed in an existing building containing other fuel-burning appliances, the room or space shall be provided with combustion air as required for new construction.
3. Sec. 507.3.1. The exception is modified to read: "Combustion air openings may be provided in an outside door provided:
(a) The door is not less than feet $\underline{2}^{\prime}$ in width and six feet eight inches $\underline{6}^{\prime} 8^{\prime \prime}$ in height; and
(b) The openings are spaced as far apart as possible or a full louvered door is provided; and
(c) The equipment room ceiling is not more than 16 inehes 16 "' above the top of the door.
4. Sec. 507.6 is added to read: "Liquefied Petroleum Gas (LPG). All provisions of this Chapter shall apply to combustion air for equipment using liquefied petroleum gas. Exceptions:
(a) The bottom of the lower combustion air opening shall be located at or below the floor of the room containing LPG-fueled equipment.
(b) The lower combustion air duct, when used, shall have the bottom installed level or with a downward slope, starting from a point at or below the equipment room floor and continuing to the outside of the structure. The duct shall terminate above the finish grade, and shall be provided with wire mesh screen to cover the opening.
(c) No pockets or trapped sections shall be permitted in any lower combustion air duct.
5. Table $5+\underline{2}$ Size of Combustion Air Openings or Ducts for Gas- or Liquid-Burning Water Heaters is modified. Refer to Table A - Size of Combustion Air Openings or Ducts for Gas- or Liquid-Burning Water Heaters.

## Table A. Size of Combustion Air Openings or Ducts for Gas- or Liquid-Burning Water Heaters

| Btu | $\underline{\text { Watts }}$ |
| :--- | ---: |
| $\underline{1000}$ | $\underline{293}$ |
| $\underline{2000}$ | $\underline{586}$ |
| $\underline{4000}$ | $\underline{1172}$ |
| $\underline{\underline{5000}}$ | $\underline{1465}$ |
| $\underline{100,000}$ | $\underline{29,300}$ |


| Column 1 <br> Existing Buildings of Ordinary Tightness | Column 2 <br> New Buildings and Unusually Tight Construction <br> Condition$\quad$ Size of Opening or Duct |  | Condition |
| :--- | :--- | :--- | :--- |

## Notes Footnotes:

${ }^{+} \underline{1}$ For location of opening, see Section 507.3.
${ }^{z} \underline{2}$ As defined in Chapter 2.
${ }^{3} \underline{3}$ When the total btu/h input rating of all enclosed appliances/equipment exceeds $100,000 \mathrm{btu} / \mathrm{h}$, the combined net free area of all combustion air openings shall be increased by not less than + one additional square inch ( 645 mm ) for each $1000 \mathrm{btu} / \mathrm{h}$ in excess of 100,000 btu/h.
${ }^{4} \underline{4}$ As defined in Section 202.0.
6. Sec. 517.6 is modified to read: No venting system shall terminate less than three feet $\underline{3}^{\prime}(0.9 \mathrm{~m})$ above any forced air inlet or evaporative cooler located within 10 feet $10^{\prime}(3.0 \mathrm{~m})$ or less than four feet $\underline{4}^{\prime}(1.2 \mathrm{~m})$ from any property line except a public way.

## R4-48-106. Chapter 6, Water Supply and Distribution

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 601.1. Add exception to read: Potable running water is not required for waterless urinals that have been approved by the Administrative Authority.
2. Sec. 603.4.18 through 603.4.18.5 are deleted and replaced with A.R.S. Title 41, Chapter 16, Article 3.
Z.3. Sec. 603.4 603.4.23 is added to read: Secondary Backflow Protection. The following occupancies shall have Reduced Pressure Principle Backflow Prevention Assemblies installed as near as practical to the water service meter connection: Hospitals, surgical clinics, laboratories, morgues, mortuaries, veterinary hospitals, industrial occupancies, packing plants, slaughter houses, chemical plants, municipal waste treatment facilities, and construction water services. NOTE: Multiple water services which are interconnected onsite and which are served from separate water mains, shall be provided with not less than a Double Check Valve Assembly at each service connection. Batteries of $\underline{\text { meters that are interconnected onsite, but which are served by the same water main, may be provided with a Single }}$

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Check Valve at each service connection.
3. Sec. 604.1 is modified to read: Water pipe and fittings shall be of brass, copper, cast iron, galvanized malleable iren, galvanized wrought iron, galvanized steel, or other approved materials. Asbestos-cement, CPVC, PE, PEX, or PVC water pipe manufactured to recognized standards may be used for cold water distribution systems outside a building. CPVC or PEX water pipe and tubing may be used for hot and cold water distribution systems within a building. All materials used in the water stpply system, except valves and similar devices shall be of a like material, except where otherwise approved by the Administrative Authority.
4. Sec. 604.11 is added to read: Cross linked polyethylene (PEX) tubing shall be marked with appropriate designation or designations consistent with the fitting system or systems for which the tubing has been listed or approved. PEX tubing shall be installed with mechanical joints in compliance with the appropriate standards and the manufacturer's instructions.
5.4. Sec. 604.12604 .13 is added to read:

Crosslinked Polyethylene-Aluminum-Crosslinked Polyethylene (PEX-AL-PEX) and Polyethylene-Aluminum-Polyethylene (PE-AL-PE) composite piping shall be marked with appropriate designation or designations consistent with the fitting system or systems for which the piping has been listed or approved. PEX-AL-PEX and PE-AL-PE piping shall be installed with mechanical joints in compliance with the appropriate standards and the manufacturer's instructions.
6.5. Sec. 608.5 is modified to read: All relief Relief valves shall be provided with a drain, not smaller than the relief valve outlet, of galvanized steel, hard drawn copper piping and fittings, $\mathrm{CPVC} PB,$, or listed relief valve drain tube with fittings which shall not reduce the internal bore of the pipe or tubing (straight lengths as opposed to coils) and shall extend from the valve to the outside of the building with the end of the pipe not more than two feet $\underline{2}^{\prime}(0.61 \mathrm{~m})$ nor less than six inches 6 " $(152.4 \mathrm{~mm})$ above the ground or the flood level of the area receiving the discharge and pointing downward. Such drains may terminate at other approved locations. No part of such drain pipe shall be trapped or subject to freezing. and The terminal end of the drain pipe shall not be threaded.
7. Move Sec. 103.5.1.1 to a new See. 610.15.
8. Move Sec. 103.5.3.3 to a new Sec. 610.16.

## R4-48-107. Chapter 7, Sanitary Drainage

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Section 701.1.2 is deleted.
2. Sec. 707.4 is modified to read: Each horizontal drainage pipe shall be provided with a cleanout at its upper terminal and each run of piping, which is more than 100 feet $100^{\prime}(30.4 \mathrm{~m})$ in total developed length, shall be provided with a cleanout for each 100 feet $\underline{100^{\prime}}$ ( 30.4 m ), or fraction thereof, in length of such piping. Exceptions:
(a) Cleanouts may be omitted on a horizontal drain line less than five feet $5^{\prime}(1.5 \mathrm{~m})$ in length unless such line is serving sinks or urinals.
(b) Cleanouts may be omitted on any horizontal drainage pipe installed on a slope of 72 degrees $\underline{72^{\circ}}$ or less from the vertical angle (angle of $1 / 5$ bend).
(c) An approved type of two-way cleanout fitting, installed inside the building wall near the connection between the building drain and building sewer or installed outside of a building at the lower end of a building drain and extended to grade, may be substituted for an upper terminal cleanout.
3. Sec. 710.6 , paragraph 1 , is modified to read: Backwater valves, gate valves, fullway ball valves, unions, motors, compressors, air tanks, and other mechanical devices required by this Section shall be located where they will be accessible for inspection and repair at all times.
4. Delete Sections 712.4, 712.4.1, 712.4.2, 712.4.3, 712.4.4.
5.4. Sec. 713.4 is modified to read: The public sewer may be considered as not being available only when so determined by the Administrative Authority (local, county, or state).
6.5 . Sec. 723.0 is modified to read: Building sewers shall be tested by plugging the end of the building sewer at its points of connection with the public sewer or private sewage disposal system and completely filling the building sewer with water from the lowest to the highest point thereof or by approved equivalent low pressure air test, or by such other test as may be prescribed by the Administrative Authority. The building sewer shall be watertight at all points. Exception: Sewer tests may be waived at the discretion of the Administrative Authority.

## R4-48-108. Chapter 8, Indirect and-Speeial Wastes

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 807.4 is modified to read: The discharge pipe of a domestic dishwashing machine may be directly connected to the tailpiece of a sink drain, or into the waste "boss" of a food waste disposer without installation of an airgap fitting. The dishwasher discharge line shall be securely fastened as high as possible, but not lower than inehes $\underline{2 "}$ ( 50.8 mm ) below the flood rim of the sink.
2. Sec. 807.5 is modified to read: No dishwashing machine utilized for commercial purposes may be directly connected
to a drainage system. Commercial dishwashers shall discharge into an approved receptor (floor sink) through an airgap of not less than ene ineh 1 ".

## R4-48-109. Chapter 9, Vents

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 903.1.2 is deleted.
2. Sec. 909.0 is modified to read:

Special Venting
909.1 Air Admittance Valve
909.1.1 Vent systems utilizing air admittance valves shall comply with this Section. Individual and branch-type air admittance valves shall conform to ASSE Standard 1051, which is incorporated by this reference and published by the American Society of Sanitary Engineering for Plumbing and Sanitary Research, 28901 Clemens Rd., Ste. 100, Westlake, OH 44145. This incorporation by reference does not include any later amendments or editions. This device shall close by gravity and seal the vent terminal at zero differential pressure (no flow condition) and under positive internal pressure. The air admittance valve provides a method of allowing air to enter the plumbing drainage system without the use of a vent extending to the open air and prevents sewer gases from escaping into the building.
909.1.2 The valves shall be installed in accordance with this Section and the manufacturer's installation instructions. Air admittance valves shall be installed after the drain, waste, and vent testing required by Sections 712.2 or 712.3 has been approved by the administrative authority.
909.1.3 Individual and branch vents shall be permitted to terminate with a connection to the air admittance valve. The air admittance valve shall only be permitted to vent fixtures on the same floor, which connect to a building drain.
909.1.4 The air admittance valve shall be located at least $4 "(102 \mathrm{~mm})$ above the horizontal branch drain, or fixture drain being vented, within the maximum developed length permitted for the vent, and shall terminate at least 6 " ( 152 mm ) above insulation materials.
909.1.5 The air admittance valve shall be accessible for the purpose of maintenance or replacement. The valve shall be located within a space that allows air to enter the valve.
909.1.6 The air admittance valve shall be rated for the size of the vent to which it is connected.
909.1.7 Within each plumbing system utilizing air admittance valves, a minimum of one vent stack shall extend outdoors to the atmosphere.
909.1.8 Air admittance valves shall not be installed in special waste systems, as described in Chapter 8, nor in spaces used as supply or return air plenums.
909.2 Island Fixtures

Traps for island sinks and similar equipment shall be roughed in above the floor and may be vented by extending the vent as high as possible, but not less than the drainboard height and then returning it downward and connecting it to the horizontal sink drain immediately downstream from the vertical fixture drain. The return vent shall be connected to the horizontal drain through a wye-branch fitting and shall, in addition, be provided with a foot vent taken off the vertical fixture vent by means of a wye-branch immediately below the floor and extending to the nearest partition and then through the roof to the open air or may be connected to other vents at a point not less than $6 "(152.4 \mathrm{~mm})$ above the flood level rim of the fixtures served. Drainage fittings shall be used on all parts of the vent below the floor level and a minimum slope of $1 /$ 4 " per foot $(21 \mathrm{~mm} / \mathrm{m})$ back to the drain shall be maintained. The return bend used under the drain board shall be a one piece fitting or an assembly of a $45^{\circ}$, a $90^{\circ}$, and a $45^{\circ}$ elbow in the order named. Pipe sizing shall be as elsewhere required in this Code.
3. Sec. 910.7 is modified to read: No water closet or urinal shall be installed on any such system. Other one, two, or three unit fixtures remotely located from the sanitary system and adjacent to a combination waste and vent system may be connected to such system in the conventional manner by means of waste and vent pipes of regular sizes, providing that the two pipe size increase required in Section 910.4 is based on the total fixture unit load connected to the system.

Note: See Appendix B of this Code for explanatory notes on the design of combination waste and vent systems.
See also Appendix L, Alternate Plambing Systems, for sizing vent piping systems.

## R4-48-110. Chapter 10, Traps and Interceptors

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 1005.0 is modified to read: Trap Seals. Each fixture trap shall have a liquid seal of not less than inehes 2 " $(50.8 \mathrm{~mm})$ and not more than four inches $4 "(101.6 \mathrm{~mm})$ except where a deeper seal is found necessary by the Administrative Authority for special conditions. Traps shall be set true with respect to their liquid seals and, where necessary, they shall be protected from freezing.
2. Sec. 1007.0 is modified to read: Trap Seal Protection. Floor drain or similar traps directly connected to the drainage system and subject to infrequent use shall be provided with an approved means of maintaining their water seals, except where not deemed necessary for safety or sanitation by the Administrative Authority. When automatic trap

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priming devices are installed, they shall be accessible for maintenance.
3. Sec. 1007.1 is added modified to read: Approved Means of Maintaining Trap Seals. Approved means of maintaining trap seals include the following, but are not limited to the methods cited:
(a) Listed Trap Seal Primer.
(b) A keyed hose bibb or bibbs within the room.
(c) Drainage from untrapped lavatories discharging to the tailpiece of those fixture traps which require priming. All fixtures shall be in the same room and on the same floor level as the trap primer.

## R4-48-111. Chapter 11, Storm Drainage

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
Sec. 1101.3 is modified to read: Material Uses. Rainwater piping placed within the interior of a building or run within a vent or shaft shall be of cast iron, galvanized steel, wrought iron, brass, copper, lead, Schedule 40 ABS DWV, Schedule 40 PVC DWV, or other approved materials, and changes in direction shall conform to the requirements of Section 706.0. ABS and PVC DWV piping installations shall be installed in accordance with IS 5, IS 9 and Chapter 15 "Firestop Protection for DWV and Stormwater Application". Except for individual single family dwelling units, materials exposed within ducts or plenums shall have a flame-spread index of not more then 25 and a smoke-developed index of not more than 50, when tested in accordance with the Test for Surface - Burning Characteristics of the Building Materials (See the Building Code standards based on ASTM E-84 and ANSI/UL 723.).

## R4-48-112. Chapter 12, Fuel Piping

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Sec. 1210.1 is modified to read:

All pipe used for the installation, extension, alteration, or repair of any gas piping shall be standard weight wrought iron or steel (galvanized or black) or yellow brass (containing not more than 75 percent $75 \%$ copper) or listed corrugated stainless steel tubing systems for interior use only-, Approved approved PE pipe may be used in exterior buried piping systems. Exception: CSST piping installed outside a building shall be sleeved, or shall be installed with protection from mechanical damage equivalent of that required for CSST piping within the building. When installed underground CSST shall be sleeved and shall have a minimum burial depth of 48 inehes $\underline{18 "}(457.2 \mathrm{~mm})$.
2. Sec. 1211.31211 .4 modify the exception to read:

Exception: When necessary due to structural conditions, approved type gas piping may be installed in other locations, when permission has first been obtained from the administrative authority. Natural gas piping (L.P. gas piping excluded) may be installed under a slab in accordance with Illustration A when building conditions prevent aboveslab installation.

## R4-48-113. Chapter 13, SpeeialPiping and-Storage-Systems Health Care Facilities and Medical Gas and Vacuum Systems (NFPA 99C, 2002 Edition)

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows: Incorporate by reference the National Fire Protection Association (NFPA) 99 Standard for Health Care Facilities (2002 Edition) and NFPA 99C Standard on Gas and Vacuum Systems ( 2002 Edition). This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from NFPA, 11 Tracy Drive, Avon, MA, 02322 and are on file with the Arizona Uniform Plumbing Code Commission.

## R4-48-114. Chapter 14, Mandatory Referenced Standards

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
Table 14-1. Add standards in alphabetical order listed under appropriate sections. Refer to Table A Plumbing Materiat standards. These standards are incorporated by reference. This incorporation by reference does not inelude any later amendments or editions. Copies of the ineorporated material are on file with the Arizona Uniform Plumbing Code Commission and the Office of the Secretary of State.
Add three new rows to Table $A$. Refer to Table $\Lambda$ Plumbing Material Standards.

Table A. Plumbing MaterialStandards Repealed

| Materiats and Products | ANSI | ASTM | FS | HАРMӨ | Other Standards | Footnote Remarks |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| NONMETALLIC PIPE: <br> Crosslinked Polyethylene-Aluminum-CrosslinkedPolyethylene (PEX AL PEX) Pressure Pipe |  | F1281 01e1* |  | C3388 | NSF 14 <br> NSF 61 |  |
| Polyethylene-Aluminum-Polyethylene (PE-AL-PE) Pressure Pipe |  | F1282-02e1* |  | C-3389 | NSF 14 <br> NSF 61 |  |
| Metal insert fittings for Polyethylene-Altuminum-Polyethylene (PE AL PE) and Crosslinked Polyethylene-Aluminum-Crosslinked Polyethylene (PEX-AL-PEX) Composite Pressure Pipe |  | F1974-01e1 * |  | C-3846 | NSF 14 <br> NSF 61 |  |
| NONMETALLIC PIPE: <br> Metal insert fittings utilizing a copper crimp ring for SDR9 Cross-Linked Polyethylene (PEX) tubing. <br> Cold Expansion Fitting with PEX reinforeing ring for use with SDR 9 Cross Linked Polyethylene (PEX Tubing) |  | F1807-97 F1960-99 |  |  |  |  |
| PLUMBING FIXTURES: <br> Waterless Urinals | Z124.9 |  |  | C 3346 |  |  |
| Note: <br> * Published by the American Society for Testing and Materials, 100 Barr Harbor Drive, West Conshohocken, PA 19428-2959. |  |  |  |  |  |  |

Incorporate by reference the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code 2000 Edition. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O., 5001 E. Philadelphia St., Ontario, CA 91761 and are on file with Arizona Uniform Plumbing Code Commission.

## R4-48-121. Appendix C, Minimmm Plumbing Faeilities Sizing of Category I Venting Systems

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
The third paragraph of Appendix $C$ is added to read: "Those jurisdictions that have not adopted a building code which stiput lates minimum plumbing facilities shall utilize Appendix C of the 1994 UPC when establishing plumbing facility requirements.
Incorporate by reference the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code 2000 Edition. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O., 5001 E. Philadelphia St., Ontario, CA 91761 and are on file with Arizona Uniform Plumbing Code Commission.

## R4-48-122. Appendix D, Rainwater Systems Reserved

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Appendix D 1 (a) is modified to read:

Rainwater piping placed within the interior of a building or run within a vent or shaft shall be of east iren, galvanized steel, wrought iron, brass, copper, lead, Schedule 40 ABS DWV, Schedule 40 PVC DWV, or other approved materials, and changes in direction shall conform to the requirements of Section 706.0.
2. Appendix D1.1(c) is modified to read: Roof drains, overflow drains, and rainwater piping installed within the building shall be tested in conformity with the provisions of this Code for testing drain, waste, and vent systems.
3. Appendix D3.3 is modified to read: Horizontal Rainwater Piping. Horizontal Rainwater Piping shall be sized in accordance with Table D-2. Exception: The potential head of water which may rise in the vertical drain pipe (tailpiece) may be used to reduce the herizontal pipe size and its slope if the head (rise) is sufficient when caleulated as follows:
(a) If the head (' $h$ ') is equal to or greater than $3 / 8$ inch for each foot ( $31.35 \mathrm{~mm} / \mathrm{m}$ ) of horizontal pipe length, the horizontal pipe may be pitched at $1 / 8$ inch slope $(10.45 \mathrm{~mm} / \mathrm{m})$, but sized according to the $1 / 2$ inch slope $(41.8 \mathrm{~mm} /$ m) table.
(b) If the head ('h') is equal to or greater than $1 / 8$ inch for each foot $(10.45 \mathrm{~mm} / \mathrm{m})$ of horizontal pipe length, the horizontal pipe may be pitched at $1 / 8$ inch slope $(10.45 \mathrm{~mm} / \mathrm{m})$, but sized according to the $1 / 4$ inch slope $(20.9 \mathrm{~mm} /$ m ) table. (See Illustration $\Lambda$ ).
EXAMPLE \#1: Roof Area - 4800 Square Feet (445.9 m 2)
Maximmm Rainfall/Hour Six Inches ( $152.4 \mathrm{~mm} / \mathrm{h}$ )
Pipe Laid at $1 / 8$ ineh Slope $(10.45 \mathrm{~mm} / \mathrm{m})$

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Using the $1 / 2$ inch slope $(41.8 \mathrm{~mm} / \mathrm{m})$ table, the horizontal pipe size will be six inches.
The available static head (' h ') needed to allow use of the $1 / 2$ inch ( $41.8 \mathrm{~mm} / \mathrm{m}$ ) table is caleulated as follows:
$3 / 8$ inch of head presstre per foot $(31.35 \mathrm{~mm} / \mathrm{m})$ of horizontal pipe run becomes $3 / 8$-ineh x 100 feet - $300 / 8$ ths, or 'h' $-371 / 2$ inches ( 952.5 mm ). NOTE: Sizing from the $1 / 8$ inch ( 10.45 mm per m) table would have required the horizontal pipe size to be eight inches $(203.2 \mathrm{~mm})$, rather than the six inches $(152.4 \mathrm{~mm})$ made possible by use of the $1 / 2$ inch $(41.8 \mathrm{~mm} / \mathrm{m})$ slope table.
EXAMPLE \#2: Roof Area 6000 Square Feet Maximum Rainfall/Hour six Inehes $(152.4 \mathrm{~mm})$ Pipe Laid at 14 8 inch Slope ( $10.45 \mathrm{~mm} / \mathrm{m}$ )
Using the $1 / 4$ ineh slope $(20.9 \mathrm{~mm} / \mathrm{m})$ table the horizontal pipe size will be eight inehes ( 203.3 mm ). The available static head (' h ') needed to allow use of the $1 / 4$ inch ( $20.9 \mathrm{~mm} / \mathrm{m}$ ) table is calculated as follows: $1 / 8$ inch of head pressure per foot ( 10.45 mm per m ) of horizontal pipe run becomes $1 / 8$-inch x 100 feet $=100 / 8$ ths, or 'h' $=121 / 2$ inches ( 317.5 mm ). NOTE: Sizing from $1 / 8$ inch ( 10.45 mm per m) table would have required the horizontal pipe size to be 10 inches ( 254.0 mm ) rather than the eight inches ( 203.2 mm ) made possible by use of the $1 / 4$ inch slope $(20.9 \mathrm{~mm} / \mathrm{m})$ table.
(c) If the head (' $h$ ') is equal to or greater than 10 feet ( 3.05 m ) (for example, base of a stack), all horizontal pipe downstream of any such vertical section may be the same size as the vertical pipe to which it is connected.

## Illustration A. Horizontal Rainwater Piping Repealed



R4-48-123. Appendix E, Manufactured of_/ Mobile Home Parks and Recreational Vehicle Parks
This appendix has no modifications.

## R4-48-124. Appendix F, Medienl-Gas-Systems Reserved

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:

1. Appendix F Medical Gas Systems. Current language is deleted and replaced with the following sections of NFPA \#99, Health Care Facilities ( 1996 Edition) as amended by this section, which are incorporated by reference. The incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from the National Fire Protection Association, Batterymareh Park, Quiney, MA 02169, and are on file with the Office of the Secretary of State:
F1 Scope
(a) The provisions herein shall apply to the installation, testing, and certification of medical gas and vacumm piping for safe use in patient care hospitals, clinies, and other health care facilities.
(b) The purpose of this appendix is to provide minimum requirements for the installation, testing, and certification of medical gas and medical vactum systems, from the point of supply to the user outlets or inlets. These provisions do no cover portable systems or cylinder storage requirements.
F2 All medical gas and vacumm piping systems are to be installed and inspected based upon applicable language found in the following chapters of NFPA \#99, Health Care Facilities (1996 edition):
Chapter 1. Introduction.

1-1 Seope<br>1-2 Applieation. (Use first paragraph; delete second paragraph)<br>1-3 Intended Use<br>1-4 Discretionary Powers of Authority Having Jurisdiction<br>$1-5$ Interpretations<br>1-6 Organization of This Doeument (subsections 1-6.1, 1-6.2, and 1-6.3)<br>1-7 Metric Units

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4-8 Effective Date
1-9Preface
Add Sec. 1-10 to read: Sections of NFPA 99, 1996 edition, which are not referenced are not mandated by the Arizona Uni-
    form Plumbing Code.
Chapter 2. Definitions.
    z 1-Official NFPA Definitions
    2 2 Definitions of Terms Used in the Standard
Chapter 4.Gas and Vacelmm Systems.
    4-3.1.1.2 Storage Requirements (Location, Construction, Arrangement)
    4-3.5 Administration - Level }
        4-3.5.1 Responsibility of Governing Body
    4-3.5.2 Gas System Policies Level4
    4-3.5.2.1 Gases in Cylinders and Liquefied Gases in Containers Level4
    4-3.5.2.2 Storage of Cylinders and Containers - Level4
    4-3.5.2.3 Patient Gas Systems - Level }
    4-3.5.3 Gas Systems Recordkeeping - Level 1
    4-3.5.4 Gas Systems Information and Warning Signs-Level1
        4-3.5.4.1 (No Heading)
        4-3.5.4.2 (No Heading)
        4-3.5.4.3 (No Heading)
    4-3.5.5 Gas System Transport and Delivery -Level 1
        4-3.5.5.1 (No Heading)
        4-3.5.5.2 (No Heading)
    4-3.5.6 Vactum Systems Policies Level4
        4-3.5.6.1 Patient Vactum Systems
    4-3.5.7 Vacuum System Recordkeeping - Level }
    4-3.5.8 Vacelm System Information and Warning Signs-Level4
        4-3.5.8.1 Piping Distribution System
        4-3.5.8.2 Gatge Identification
    4-3.5.9 WAGD System Policies - Level }
        4-3.5.9.1 Maintenance
        4-3.5.9.2 Performance Tests
    4-4 Level }2\mathrm{ Piped Systems
        4-4.1 Piped Gas Systems - Level 2
        4-4.2 Piped Vacutm Systems-Level }
        4-4.3 Piped WAGD Systems - Level }
        4-4.4 Performance Criteria and Testing Level }
        4-4.5 Administration Level }
    4-5.5 Administration-Level }
        4-5.5.1 Responsibility of Governing Body
    4-5.5.2 Gas System Policies - Level }
        4-5.5.2.1 (No Heading)
        4-5.5.2.2 (No Heading)
        4-5.5.2.3 Patient Gas Systems Level }
        4-5.5.3 Gas System Recordkeeping - Level 3
        4-5.5.4 Gas System Information and Warning Signs - Level }
        4-5.5.5 Gas System Transpert and Delivery Level 3
        4-5.5.6 Vactum System Policies Level }
        4-5.5.7 Vacutm System Recordkeeping Level }
        4-5.5.8 Vacuum System Information and Warning Signs - Level }
        4-5.5.9 WAGD System Policies - Level }
    4-6.5 Administration - Level 4
Chapter 12. Hospital Requirements.
    12-1 Seope
    12-3.4 Gas and Vacutmm System Requirements
        12-3.4.1 (No Heading)
        12-3.4.2 (No Heading)
        12 3.4.3 (No Heading)
        12-3.4.4 (No Heading)
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    12 3.4.5(No Heading)
Chapter 13. Ambulatory Health Care Center Requirements.
    13-1 Seope
    13-3.4-Gas and Vacutm System Requirements
            13-3.4.1 (No Heading)
            13-3.4.2 (No Heading)
            13-3.4.3 (No Heading)
            13-3.4.4 (No Heading)
            13-3.4.5 (No Heading)
Chapter 14. Clinic Requirements.
    14-1 Scope
    14-3.4 Gas and Vacumm System Requirements
            14-3.4.1 (No Heading)
            14-3.4.2 (No Heading)
            14-3.4.3 (No Heading)
            14-3.4.4 (No Heading)
            14-3.4.5 (No Heading)
Chapter 15. Medieal and Dental Office Requirements.
    15-1-General
            15-1.1 Scope
            15-1.2 Applicability
        15-3.4 Gas and Vacmum System Requirements
            15-3.4.1 (No Heading)
            15-3.4.2 (No Heading)
            15-3.4.3 (No Heading)
            15-3.4.4 (No Heading)
            15-3.4.5 (No Heading)
            15-3.4.6(No Heading)
            15-3.4.7 (No Heading)
            15-3.4.8 (No Heading)
            15-3.4.9 (No Heading)
Chapter 16. Nursing Home Requirements.
    16-1 Scope
    16-3.4 Gas and Vacutmm System Requirements
    16-3.4.1 (No Heading)
    16-3.4.2 (No Heading)
Chapter 17. Limited Care Facility Requirements.
    17-1 Seope
    17-3.4 Gas and Vaeumm System Requirements
            17-3.4.1 (No Heading)
            17-3.4.2 (No Heading)
Chapter 19. Hyperbaric Facilities,
    19-1 Introduction and Seope
    19-3.3.3 (No Heading)
    19-3.3.5 (No Heading)
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## R4-48-125. Appendix G, Graywater Systems for Single Family Dwellings

A. Appendix G of the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code (1994 2000 Edition) is incorporated by reference. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O. at 20001 Walmet Drive South, Walntt, CA, 91789-2825 5001 E. Philadelphia St., Ontario, CA 91761 and are on file with the Arizona Uniform Plumbing Code Commission. and the Office of the Secretary of State. Notwithstanding any other provision of R4-48-125, the provisions of R4-48-125 do not require an Administrative Authority to act in a manner that conflicts with other provisions of state law or duplicates any act required by other provisions of state law.
B. Appendix G, incorporated by subsection (A) is modified as follows:

Correct grammatically for consistency the following tables:
Table A Irrigation Disposal Fields;
Table B Location of Graywater Systems and Setback Requirements and;
Table C Effluent Application Loading Rates to Soil for Graywater Systems

Table A. Irrigation Disposal Fields
Irrigation/disposal fields shall be constructed as follows:

|  | Minimum | Maximum |
| :---: | :---: | :---: |
| Number of perforated drain lines per irrigation zone | 1 | -- |
| Length of each perforated drain line | -- | $100 \mathrm{ft} \underline{100}$ ' $(30.5 \mathrm{~m})$ |
| Bottom width of trench | 6 in. $6^{\prime \prime}(15.2 \mathrm{~cm})$ | z4 im. 24 " ( 61.0 cm ) |
| Spacing of lines, center-to-center | $4 \mathrm{ft} \underline{4}$ ( $1 / 2 \mathrm{~m}$ ) | -- |
| Depth of earth cover over aggregate | 9 -im. 9" $(22.9 \mathrm{~cm})$ |  |
| Depth of aggregate cover over the lines | $2 \mathrm{in} . \underline{2 "}(5.1 \mathrm{~cm})$ | -- |
| Depth of aggregate beneath lines | 3 in . ${ }^{\prime \prime}$ ( 7.6 cm ) | -- |
| Grade of perforated lines | Level | $\begin{aligned} & 3 \mathrm{im} . / 100 \mathrm{ft.} 3 " / 100^{\prime} \\ & (7.6 \mathrm{~cm} / 30.5 \mathrm{~m}) \end{aligned}$ |
| Total depth of trench | $17 \mathrm{in}. 17^{\prime \prime}(43.1 \mathrm{~cm})$ | $24 \mathrm{in}. 4^{\prime \prime}$ (61 cm) |

Table B. Location of Graywater System and Setback Requirements

| Minimum Horizontal Distance in Clear Required From: | Holding/Surge Tank (feet)(meters) | Irrigation/Disposal Field (feet)(meters) |
| :---: | :---: | :---: |
| Building structures (see footnote 1) | $5 \mathrm{ft} . \underline{5^{\prime}}(1.5 \mathrm{~m})$ (see footnote 2) | $2 \mathrm{ft}$. 2' $(0.6 \mathrm{~m})$ (see footnote 3) |
| Property line adjoining private property | 5 ft. 5' (1.5 m) | (see footnote 4) |
| Water supply wells (see footnote 5) | $100 \mathrm{ft} \underline{100}$ ( 30.5 m ) | 100 ft . 100 ' ( 30.5 m ) |
| Streams, lakes, and reservoirs (see footnote 5) | 100 ft . $\underline{\text { 00 }}$ ( 30.5 m ) | $\begin{aligned} & 100 \mathrm{ft} .100^{\prime}(30.5 \mathrm{~m}) \\ & (\text { see footnote } 6) \end{aligned}$ |
| Domestic water source (see footnote 7) | 200 ft 200' $(61.0 \mathrm{~m})$ | 200 ft 200' $(61.0 \mathrm{~m})$ |
| Dry wash/drainage easements (see footnote 10) | 50 ft 50' ( 15.2 m ) | 50 ft 50' $(15.2 \mathrm{~m})$ |
| Sewage pits | $5 \mathrm{ft} 5^{\prime}(1.5 \mathrm{~m})$ |  |
| Disposal field and 100\% expansion area | 5 ft . ${ }^{\prime}$ ( 1.5 m ) | 4 ft . ${ }^{\text {' }}$ (see footnote 8 ) |
| Septic tank | $\theta$ ft. $\underline{0}$ | 5 ft. 5' (1.5 m) |
| Onsite domestic water service line | $5 \mathrm{ft} \underline{5^{\prime}}$ ( 1.5 m ) | 5 ft. $\underline{5}^{\prime}(1.5 \mathrm{~m})$ |
| Pressurized public water main | $10 \mathrm{ft} \underline{10}$ ' ( 3.0 m ) | $10 \mathrm{ft} . \underline{10}$ ' $(3.0 \mathrm{~m})$ (see footnote 9) |

Netes Footnotes: When irrigation/disposal fields are installed in sloping ground, the minimum horizontal distance between any part of the distribution system and the ground surface shall be 15 feet 15 , ( 4.6 m ).

1 Including porches and steps, whether covered or uncovered, breezeways, roofed patios, carports, covered walks, covered driveways and similar structures or appurtenances.
2 The distance may be reduced to 0 feet for above-ground tanks when 1st approved by the Administrative Authority.
3 Assumes a $45^{\circ}$ angle from foundation.
4 The setback requirement is 5 feet $5^{\prime}(1.5 \mathrm{~m})$ unless the property is not served by a central system for the distribution of water and:
a. There is no existing or proposed individual well on adjoining private property, in which case the setback is 50 feet 50 ' $(15.2 \mathrm{~m})$; or
a.b. A 100 -foot $100^{\prime}(30.5 \mathrm{~m})$ separation distance cannot be maintained from an existing or proposed individual well, in which case the setback is the distance necessary to maintain the 100 -foot $100^{\prime}(30.5 \mathrm{~m})$ separation; or
b.c. The applicable setback requirement in (a) or (b) may be reduced to a minimum of 5 feet 5 ' $(1.5 \mathrm{~m})$ with a variance from the Administrative Authority.
5 Where special hazards are involved, the distance required shall be increased as may be directed by the Administrative Authority.
6 These minimum clear horizontal distances shall also apply between the irrigation/disposal field and the maximum lake or reservoir level.

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7 A point of water intake or suction pipeline from any stream, lake, or reservoir that is used for the purpose of providing water for human consumption.
8 Plus 2 feet $\underline{2}^{\prime}(0.6 \mathrm{~m})$ for each additional foot (meter) of depth in excess of foot $\underline{1}(0.3 \mathrm{~m})$ below the bottom of the drain line.
9 For parallel construction/for crossings, approval by the Administrative Authority shall be required.
1050 Fifty foot ( 15.2 m ) setback is measured from the edge of the defined natural channel bank for a drainage area of at least 5 five acres ( 2 hectare) or a drainage easement, whichever is less. Setback may be reduced to 25 feet $25^{\prime}$ ( 7.6 m ) up gradient from the system, if channel erosion protection is provided (naturally or man-made) and approved by the Administrative Authority.
Table C. Effluent Application Loading Rates to Soil for Graywater Systems
Instructions: Read questions in sequence beginning with $A$. The maximum soil loading rate in gallons per day per square foot (gpd/sq. ft.) (Lpd/sq. m.) corresponds to the first "yes" response to the questions.

|  | Soil Application Rates gpd/sq. ft. (Lpd/ sq. m.) | Percolation Rate Range minutes/inch (min./cm.) |
| :---: | :---: | :---: |
| A. Is the horizon gravelly coarse sand or coarser? | 0 (0) (see footnote 4 ) | -- |
| B. Is the structure of the horizon moderate or strongly platy? | 0 (0) (see footnote 1) | -- |
| C. Is the texture of the horizon sandy clay loam, clay loam, silty clay loam, or finer and structure weak and platy? | 0 (0) (see footnote 4 ) | -- |
| D. Is the moist consistence stronger than firm or any cemented class? | 0 (0) (see footnote 4 ) | -- |
| E. Is texture sandy clay, clay, or silty clay of high clay content and structure massive or weak? | 0 (0) (see footnote 1 ) | -- |
| F. Is texture sandy clay loam, clay loam, silty clay loam, or silty loam and structure massive? | $0(0)$ (see footnote 4 ) | -- |
| G. Is the texture of the horizon loam or sandy loam and the soil structure massive? | . 20 (8.15) | 16-45 (6.3-17.7) |
| H. Is texture sandy clay, clay or silty clay of low clay content and the structure moderate or strong? | . 20 (8.15) | 45-60 (17.7-23.6) |
| I. Is texture sandy clay loam, clay loam, or silty loam and structure weak? | . 20 (8.15) | 45-60 (17.7-23.6) |
| J. Is texture sandy clay loam, clay loam, or silty clay loam and structure moderate or strong? | . 40 (16.30) | 45-60 (17.7-23.6) |
| K. Is texture sandy loam, loam, silty loam and structure weak? | . 40 (16.30) | 16-45 (6.3-17.7) |
| L. Is texture sandy loam, loam, silty loam and structure moderate or strong? | . 60 (24.45) | 16-45 (6.3-17.7) |
| M. Is texture fine sand, very fine sand, loamy fine sand, or loamy very fine sand? | . 40 (16.30) | 10-30 (3.9-11.8) |
| N. Is texture loamy sand or sand? | . 80 (32.59) | 10-20 (3.9-7.9) |
| O. Is texture coarse sand? | 1.20 (48.89) | 2-10 (0.8-3.9) |

Notes Footnote:
4 Graywater systems for these soil types shall comply with paragraph G12(a) and G12(b) of this appendix.

## R4-48-126. Appendix H, Proedtres-for-Sizing-Commerial-Kitehen-Grens-Intereeptors Recommended Procedures for Design, Construction and Installation of Commercial Kitchen Grease Interceptors

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
Appendix H 2 (a) 103.1 is modified to read: Interceptors shall be constructed in accordance with the design approved by the Administrative Authority and shall have a minimum of two compartments with fittings designed for grease retention. Grease interceptors shall be constructed of solid durable materials, not subject to excessive corrosion or decay, and shall be watertight.

## R4-48-127. Appendix $\ddagger \underline{K}$, Private Sewage Disposal Systems

In accordance with A.R.S. §41-619(B)(4), the Arizona Uniform Plumbing Code Commission adopts Appendix I of the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code (1994 2000 Edition) as modified in Appendix A. Notwithstanding any other provision of R4-48-127, the provisions of R4-48-127 do not require an Administrative Authority to act in a manner that conflicts with other provisions of state law or duplicates any act required by other provisions of state law. Is modified as follows:

## Appendix A. Modified-Appendix I

## ¥K 1 Private Sewage Disposal - General

(a) Where permitted by Section 713.0, the building sewer may be connected to a private sewage disposal system complying with the provisions of this appendix. The type of system shall be determined on the basis of location, soil absorption rate, soil classification, and depth to the ground water below the land surface and shall be designed to receive all sewage from the property. The system, except as otherwise approved, shall consist of a septic tank with effluent discharging into a subsurface disposal field, into one or more seepage pits, or into a combination of subsurface disposal field and seepage pits. The Administrative Authority may grant exceptions to the provisions of this appendix for permitted structures which have been destroyed due to fire or natural disaster, and which cannot be reconstructed in compliance with these provisions.
(b) Where the quantity or quality of the sewage is such that the above system cannot be expected to function satisfactorily; for commercial, agricultural, and industrial plumbing systems; for installations where appreciable amounts of industrial or indigestible wastes are produced; for occupancies producing abnormal quantities of sewage or liquid waste; or when grease interceptors are required by other parts of this Code, the method of sewage treatment and disposal shall be first approved by the Administrative Authority. Special sewage disposal systems for minor, limited, or temporary uses shall be first approved by the Administrative Authority. This appendix applies only to systems with an inflow of 3000 gallons (11,355 liters) per day or less.
(c) Disposal systems shall be designed to utilize the most porous or absorptive portions of the soil formation. Where the depth to the ground water extends to within the specified minimum vertical separation for the proposed system, a private sewage disposal system shall not be installed.
(d) The minimum vertical separation from the bottom of the disposal field or seepage pit shall be as specified in Tables $\ddagger \underline{K}-4$ (A), $\ddagger \underline{K}-4$ (B), $\ddagger \underline{K}-5$ or $\ddagger \underline{K}-7$.
(e) When making a site investigation and determining the soil characterization and soil absorption rates for private sewage disposal systems and alternative private sewage treatment and disposal systems, an investigator shall use one or more of the following standards, including (1) through (7), incorporated by reference, or methods or equivalent standards approved by the Administrative Authority. The incorporated standards do not include later amendments or editions and are available from the Registrar of Contractors, and the Office of the Secretary of State:
(1) ASTM D 5879-95, "Standard Practice for Surface Site Characterization for On-Site Septic Systems";
(2) ASTM D 5921-96, "Standard Practice for Subsurface Site Characterization of Test Pits for On-Site Septic Systems";
(3) ASTM D 1452-80 (Reapproved 1995), "Standard Practice for Soil Investigation and Sampling by Auger Borings." This method shall be used in areas if the depth to groundwater may be within the required minimum vertical separation from the bottom of the subsurface disposal field for the private sewage disposal system;
(4) ASTM C1227-00, "Standard Specification for Precast Concrete Septic Tanks";
(5) IAPMO PS1-93, "Material and Property standard for Prefabricated Septic Tanks";
(6) ACI 318-99, "Building Code Requirements for Structural Concrete" and ACI 318R-99, "Commentary";
(7) ACI 350R-89, "Environmental Engineering Concrete Structures", or
(8) Percolation testing as specified in Section $\ddagger \underline{K} 15$.
(f) A site investigation shall include a log of soil formations, percentage of rock, texture, structure, consistence, and mottles as provided in ASTM D-5921-96, depth to ground water below the land surface as determined by test holes dug in close proximity to any proposed disposal field or seepage pit (for example, published groundwater data, subdivision reports, or relevant well data), soil classification, or percolation test results. Other information acceptable to the Administrative Authority may be utilized to determine soil performance equivalent to that achieved by the standards or methods specified in Section I K 1 (e).
(g) All private sewage disposal systems shall be so designed that additional seepage pits or subsurface drain fields, equivalent to at least $100 \%$ of the required original system, may be installed if the original system cannot absorb all the sewage. No division of the lot or erection of structures on the lot shall be made if such division or structure impairs the usefulness of the $100 \%$ expansion area.
(h) No property shall be improved in excess of its capacity to treat and dispose of sewage effluent by the means provided in this Code.
(i) No private sewage disposal system, or part thereof, shall be located in any lot other than the lot which is the site of the building or structure served by such system; nor shall any private sewage disposal system or part thereof, be located at any point having less than the minimum distances indicated in Table $\ddagger \underline{K}-1$.

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Nothing in this Code shall be construed to prohibit the use of all or part of an abutting lot to provide additional space for a private sewage disposal system or part thereof, when proper cause, transfer of ownership, or change of boundary not in violation of other requirements has been first established to the satisfaction of the Administrative Authority. The instrument recording such action shall constitute an agreement with the Administrative Authority which shall clearly state and show that the areas so joined or used shall be maintained as a unit during the time they are so used. Such agreement shall be recorded in the office of the County Recorder as part of the conditions of ownership of said properties, and shall be binding on all heirs, successors, and assigns to such properties. A copy of the instrument recording such proceedings shall be filled with the Administrative Authority.
(j) No building permit shall be issued until the Administrative Authority has approved the site for a private sewage disposal system.
(k) Nothing contained in this appendix shall be construed to prevent the Administrative Authority from requiring compliance with statutes, ordinances, or rules having higher requirements than those contained herein, where such statutes, ordinances, or rules are essential to maintain a safe and sanitary condition.
(l) Exception: The Administrative Authority may, at its discretion, approve an alternative private sewage treatment and disposal system.

## †K 2 Definitions

(a) Administrative Authority - A municipality or county that enforces the state plumbing code. The director of the Arizona Department of Environmental Quality may delegate functions, powers, or duties to a municipality or county under A.R.S. § 49-107.
(b) Aggregate - Clean graded hard rock or gravel. Aggregate shall have not more than $2 \%$ fines by weight. Aggregate shall be of uniform size, $3 / 4$ inches $\underline{3 / 4 "}(19.1 \mathrm{~mm})$ to $21 / 2$ inches $\underline{2} 1 / 2^{\prime \prime}(63.5 \mathrm{~mm})$ in diameter, and shall offer $30 \%$ or more void space. The aggregate shall have a hardness value of three or greater on the Moh's Scale of Hardness (aggregate that can scratch a copper penny without leaving any residual rock material on the coin would be a hardness of three or more on the Moh's Scale of Hardness). Volcanic rock that meets the above criteria may be substituted for hard rock or gravel.
(c) Bedroom - A habitable room providing privacy and used for sleeping purposes. For the purposes of this Code, a loft or a basement shall be considered a bedroom.
(d) Disposal Area - Area within the horizontal plane that is delineated by a simple figure that encompasses the soil absorption components of a wastewater system.
(e) Disposal Bed - A type of bottom area absorption system that uses an underground area up to 12 feet 12 ' ( 3.7 m ) wide, partially filled with aggregate. Piping distributes the effluent evenly throughout the entire bed.
(f) Disposal Field (Drainfield) - An aggregate-filled bed or trench into which effluent is discharged for final treatment and disposal. A soil absorption system is constructed to permit the discharge of treated sewage effluent into native soil. Construction is performed following site-specific specifications including soil excavation and the installation of disposal piping, aggregate, and other specified components and materials. The plan view of a drainfield shows the disposal area. The soil absorption area of a drainfield is the total surface within a drainfield that is approved by the Administrative Authority for the discharge of treated sewage effluent into the native soil.
(g) Disposal Pipe - Pipe that is placed in disposal trenches, beds, or a seepage pit to disperse effluent to the soil absorption surfaces.
(h) Disposal Pit (Seepage Pit) - A type of sidewall absorption system that uses a vertical, cylindrical underground excavation constructed to permit disposal of effluent by soil absorption through the pit's walls.
(i) Disposal Trench - A type of absorption trench that uses an area, excavated 1 foot $\underline{1}(.3 \mathrm{~m})$ to 3 feet $\underline{3}$ ' ( 9 m ) wide, which contains aggregate and a single effluent disposal pipe.
(j) Distribution Box - A watertight structure that receives and distributes effluent in equal portions to $z$ two or more pipes that convey effluent to disposal pipes.
(k) Domestic Water Source Intake - A point of water intake or a suction pipeline located in any stream, lake, or reservoir that is used for the purpose of providing water for human consumption.
(l) Dosing Tank - A watertight receptacle located between the treatment unit and the drainfield, equipped with a pump or siphon, that stores and delivers doses of treated sewage effluent to the drainfield.
(m) Dry Wash - A watercourse that only flows in direct response to precipitation and whose channel at all times is above the water table.
(n) Effective Absorption Area - Area of native soil that is approved by the Administrative Authority for the absorption of treated sewage effluent in a disposal trench, pit, or other approved drainfield.
(o) Failure - The inability of any disposal system component to function as designed.
(p) Five-Day Biochemical Oxygen Demand (BOD) - The quantity of oxygen used in the biochemical oxidation of organic matter in five days at 20 degrees $20^{\circ}$ Centigrade under specific conditions and reported as milligrams per liter (mg/l).
(q) Groundwater - Water that is in the zone of saturation and under pressure equal to or greater than atmospheric pressure.
(r) Impermeable layer - A soil zone with a percolation rate numerically greater than 120 minutes per inch or soils classified as impermeable (for example: clay or rock).
(s) Live stream - A watercourse with perennial flow or where surface water is present at least $10 \%$ of the time during a calendar year, based upon historic flow or weather records.
(t) Mottles - Soil color patterns caused by alternating saturated (anaerobic) and unsaturated (aerobic) soil conditions.
(u) Percolation Test - An empirical test used to estimate the rate at which effluent is absorbed by the soil.
(v) Repair - The extension, alteration, replacement, or relocation of existing components of a private sewage disposal system.
(w) Rock - A body of consolidated or partially consolidated material, composed of minerals and located at or below the land surface. Rock includes bedrock (fractured or unfractured) and partially-weathered rock that is relatively hard and cannot be dug with a hand shovel.
(x) Septage - All sludge, scum, liquid, or other material treated using a private sewage disposal system.
(y) Site - The location of an existing or proposed private sewage disposal system.
(z) Site Investigation - The practice of investigating, evaluating, and reporting on soil, topographic, and location conditions that affect the design and function of a private sewage disposal system.
(aa) Soil Evaluation - The practice of investigating, characterizing, and reporting the properties of soil used to absorb treated sewage effluent in a zone of unsaturated flow.
(ab) Soils - The naturally occurring, unconsolidated mineral and organic material on the land surface, developed from rock and other parent material that consists of sand, silt, and clay-sized particles and variable amounts of organic matter. In a zone of transition between two types of soil, the soil will be classified according to those soil characteristics that represent $51 \%$ or more of the total zone.
(ac) Soil Profile - A vertical cross-section of the undisturbed soil showing the characteristic soil horizontal layers or soil horizons that have formed as a result of the combined effects of parent material, topography, climate, biological activity, and time.
(ad) Total Suspended Solids (TSS) - Solids in wastewater that can readily be removed by standard filtering procedures in a laboratory and reported in milligrams per liter ( $\mathrm{mg} / \mathrm{l}$ ).

## £ K 3 Capacity of Septic Tanks

The design liquid capacity of all septic tanks shall conform to Table $\ddagger \underline{K}-2$ and $\ddagger \underline{K}-3$.

## †K $\mathbf{4}$ Area of Disposal Fields

The minimum effective absorption area in disposal fields and estimated waste/sewage flow rate shall conform to Tables $\ddagger \underline{K}-3$, $\mp \underline{K}-4(\mathrm{~A}), \mp \underline{K}-4(\mathrm{~B})$ and $\mp \underline{K}-7$ and shall be as follows:
(1) When disposal fields are installed, a minimum of 150 square feet ( $13.9 \mathrm{sq} . \mathrm{m}$ ) of trench bottom shall be provided for each system exclusive of any hard pan, rock, clay, or other impervious formations. Sidewall area in excess of the required 12 inches $12 "(30.5 \mathrm{~cm})$ and not to exceed 36 inches $\underline{36 "}(91.4 \mathrm{~cm})$ below the disposal pipe may be added to the trench bottom area when computing absorption areas.
(2) When leaching beds are installed in lieu of trenches, the area of each such bed shall be at least $50 \%$ greater than the tabular requirements for trenches. Perimeter sidewall area in excess of the required 12 inehes $12 "(30.5 \mathrm{~cm})$ and not to exceed 36 inches $36^{\prime \prime}(91.4 \mathrm{~cm})$ below the disposal pipe may be added to the trench bottom area when computing absorption areas.
(3) No excavation for a disposal pipe or disposal bed shall extend within the system's specified minimum vertical separation in order to ensure the system does not contaminate the underlying groundwater in excess of Arizona Aquifer Water Quality standards.
(4) When leaching chambers are installed in lieu of pipe and aggregate, an equivalent absorption area shall be provided based on the calculated effective chamber absorption area. The calculated effective chamber absorption area is the nominal open-bottom absorption area (length times width) times 1.43 , plus the product of two times the vertical height of the sidewalls times the chamber length. The sidewall chamber shall provide a minimum of $35 \%$ open area for side wall credit to be allowed, and shall be constructed to minimize the movement of fines into the chamber area. The use of filter fabric or geotextile against sidewall openings is prohibited. The required minimum absorption area shall be calculated using table Table $\ddagger \underline{K}-4$ (A), $\ddagger \underline{K}-4$ (B) or $\ddagger \underline{K}-7$.
Example:
The chamber to be used has an open bottom 3 feet $\underline{3}$ ' wide, 6 feet $\underline{6}$ ' long, and has 4 one vertical foot ( $0.9 \mathrm{~m}, 1.8$ m , and 0.3 m deep) of sidewall. The disposal system is for a 3 bedroom three bedroom dwelling. The soil is loamy sand $(\mathrm{N})$. Depth of chamber bottom is to be less than five feet $\underline{5}^{\prime}(1.5 \mathrm{~m})$ below the finished grade (the installation is considered a shallow system).
The calculated effective chamber absorption area per chamber is:
Chamber bottom area $=3$ feet 3 ' $\times 6$ square feet ( $0.9 \times 1.8 \mathrm{sq} . \mathrm{m}$.) of open bottom area $\times 1.43=25.74$ square feet ( 2.39 sq. m.), plus
Chamber sidewall $=z$ two sidewalls $\times 4$ foot $\underline{1}^{\prime}$ high $\times 6$ feet $\underline{6^{\prime}}$ long $=12$ square feet $(2 \times 0.30 \mathrm{~m} . \times 1.83 \mathrm{~m} .=$ 1.11 sq. m.).

The effective chamber absorption area $=$ the chamber bottom plus sidewalls

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> Chamber sidewall $=25.74$ square feet $(2.39 \mathrm{sq} . \mathrm{m})+$.12 square feet $(1.11 \mathrm{sq} . \mathrm{m})=$.37.74 square feet $(3.5 \mathrm{sq} . \mathrm{m}$. per chamber.
> The number of chambers needed is calculated as follows:
> Wastewater flow rate is three bedrooms x 150 gallons per day $(568 \mathrm{lpd})$, or 450 gallons per day $(1703 \mathrm{lpd})$. The soil application rate for loamy sand [Table $\pm \underline{\mathrm{K}}-4(\mathrm{~A})$, Question N, Column A$]$ is listed as 0.80 gallons per day per square foot $(32.6 . \mathrm{lpd} / \mathrm{sq} . \mathrm{m}$.$) Dividing the flow rate, 450$ gallons per day $(1703 \mathrm{lpd})$, by the soil application rate, 0.80 gpd $/ \mathrm{sq} . \mathrm{ft} .(32.6 \mathrm{lpd} / \mathrm{sq} . \mathrm{m}$.$) yields a total absorption area of 562.5$ square feet $(52.3 \mathrm{sq} . \mathrm{m}).$. Since the effective chamber absorption area of each chamber is 37.74 square feet $(3.5 \mathrm{sq} . \mathrm{m}$.$) , a total of 14.9$ chambers are needed.
> Required area $=562.5$ square feet $(52.3$ sq. m. $)$ divided by 37.74 square feet $(3.5 \mathrm{sq} . \mathrm{m}$.$) .$
> Round up to a total chamber requirement of 15 chambers.

## ¥K 5 Area of Seepage Pits

The minimum effective absorption area in any seepage pit shall be predicated on estimated waste/sewage flow rates in Table $\ddagger$ K-3 and shall conform to Tables $\ddagger \underline{K}-5$ and $\ddagger \underline{K}-6$ as follows:
(1) The minimum effective absorption area in any seepage pit shall be calculated as the excavated sidewall area below the inlet exclusive of any hardpan, rock, clay, or other impervious formations.
(2) Seepage pit sizes may be computed from Table $\ddagger \underline{K}-5$ or using percolation tests prescribed in $\ddagger \underline{K} 15$.
(3) The minimum required area of porous formation shall be provided in one or more seepage pits. No seepage pit excavation shall extend into the system's specified minimum vertical separation from the water table nor to a depth where sewage may contaminate the underlying groundwater that is protected by state law for domestic or drinking water purposes.
(4) The applicant shall supply acceptable evidence of depth to groundwater to the Administrative Authority.
(5) A boring log that describes soil from the seepage pit shall be submitted to the Administrative Authority.

## I K 6 Soil Testing

Seepage pit and disposal field sizes shall be computed from Tables $\ddagger \underline{K}-4$ (A), $\ddagger \underline{K}-4$ (B), $\ddagger \underline{K}-5, \mp \underline{K}-6$, and $\ddagger \underline{K}-7$.

## £ K 7 Septic Tank Design and Construction

(a) All septic tanks shall meet the specifications set forth in $\ddagger \underline{K} 7$, (b) through (q).
(b) Septic tank designs shall produce a clarified effluent and shall provide adequate space for sludge and scum accumulations.
(c) Septic tanks shall be constructed of solid durable materials, not subject to excessive corrosion or decay and shall be watertight.
(d) Septic tanks shall have a minimum of two compartments except when placed in series. The inlet compartment of any septic tank not placed in series shall be nominally 67 to $75 \%$ of the total required capacity of the tank. Septic tanks placed in series shall be considered as a unit and shall meet the same criteria as a single tank. The liquid depth of the septic tank shall not be less than 42 inches $42 "(1.07 \mathrm{~m})$. A septic tank of 1000 gallon capacity shall have a length of at least 8 feet $\underline{8}$ $(2.44 \mathrm{~m})$. For septic tanks of greater capacity, the tank length shall be at least two times but not more than three times the width.
(e) Access to each septic tank interior shall be provided by at least two access openings 20 inehes 20 " ( 50.8 cm ) in minimum dimension. One access opening shall be located over the inlet and one access opening shall be located over the outlet. Whenever a first compartment exceeds 12 feet $12^{\prime}(3.7 \mathrm{~m})$ in length, an additional access opening shall be provided over the baffle wall. Access openings and risers, if needed, shall be constructed to ensure accessibility within 6 inches $\underline{6 "}$ ( 0.15 $\mathrm{m})$ below grade. A permanent surface marker appropriate to the site shall be provided for locating the septic tank access openings for maintenance.
(f) The inlet and outlet pipe openings shall be not less in size than the connecting sewer pipe. The vertical leg of a round inlet and outlet fittings shall not be less in size than the connecting sewer pipe nor less than 4 inches $\underline{4 \prime \prime}(10.1 \mathrm{~cm})$. A baffle-type fitting shall have the equivalent cross-sectional area of the connecting sewer pipe and not less than a 4 inehes $\underline{4 "}(10.2 \mathrm{~cm})$ horizontal dimension when measured at the inlet and outlet pipe inverts.
(g) The inlet and outlet pipe or baffle shall extend 4 inches 4 " $(10.2 \mathrm{~cm})$ above and at least 12 inehes 12 " ( 30.5 cm ) below the water surface. The invert of the inlet pipe shall be at a level not less than $\mathcal{Z}$ inehes $\underline{2 "}(5.1 \mathrm{~cm})$ above the invert of the outlet pipe.
(h) Inlet and outlet pipe fittings or baffles, and compartment partitions shall have a free vent area equal to the required crosssectional area of the house sewer or private sewer discharging therein to provide free ventilation above the water surface from the disposal field or seepage pit through the septic tank, house sewer, and stack to the outer air.
(i) The sidewalls shall extend at least 12 inehes $\underline{12 "}(30.5 \mathrm{~cm})$ above the liquid depth. The cover of the septic tank shall be at least 2 inches $\underline{2 "}(5.1 \mathrm{~cm})$ above the top of the inlet fitting vent opening.
(j) Partitions or baffles between compartments shall be of solid, durable material and shall extend at least 4 inches 4" (10.1 cm ) above the liquid level. The open area of the baffle shall be between one and two times the open area of the inlet pipe or for a horizontal slot, shall be no more than 6 inehes 6 " in height, and shall be located at the midpoint of the liquid level of the baffle. Wooden baffles are prohibited.
(k) Each tank shall be structurally designed to withstand all anticipated earth or other loads. All septic tank covers shall be capable of supporting an earth load of 300 pounds per square foot $(14.4 \mathrm{kPa})$ for a minimum soil cover of Z feet $\underline{2}^{\prime}(0.61$ $\mathrm{m})$. When the top of the tank is greater than 2 feet $\underline{2}^{\prime}(0.61 \mathrm{~m})$ below finished grade, the septic tank and cover shall be capable of supporting an additional load of 150 pounds per square foot $(7.2 \mathrm{kPa})$ for each additional foot of cover.
(l) Septic tanks installed under concrete or black top paving shall have the required access openings extended to grade in a manner acceptable to the Administrative Authority.
(m) The inlet and outlet ends of the tank shall be clearly and permanently marked on the outside of the tank with the terms "INLET" or "IN," and, "OUTLET" or "OUT," above, or to the right or left of the corresponding inlet and outlet openings.
(n) It is permissible to have septic tanks placed in series to meet the minimum septic tank capacity requirements.
(o) Materials
(1) Cast in Place Concrete Septic Tanks

All concrete septic tanks shall be protected from corrosion by coating with an approved bituminous coating by construction with a concrete mix incorporating $15 \%$ to $18 \%$ fly ash, or by other acceptable means. The coating shall extend to at least 4 inehes $\underline{" \prime}(101.6 \mathrm{~mm})$ below the water line, and shall cover all of the internal area above that point. Septic tanks constructed in place shall comply with the American Concrete Institute (ACI) standards 318-99, 318R99, and 350R-89.
(2) Steel Septic Tanks

The minimum wall thickness of any steel septic tank shall be No. 12 U.S. gauge ( 0.109 cm ) and each tank shall be protected from corrosion, both externally and internally, by an approved bituminous coating or by other acceptable means.
(3) Prefabricated septic tanks

Materials for precast concrete septic tanks shall comply with ASTM C1227-00. Materials for fiberglass or polyethylene septic tanks shall comply with IAPMO PS1-93. If any conflict exists between this appendix and ASTM C122700 or IAPMO PS1-93, the requirements of this appendix shall apply.
(4) Alternative materials

Septic tanks constructed of alternative materials may be approved by the Administrative Authority if they comply with approved, applicable standards in this Code.
(5) Prohibited Materials

Wooden, block, and bare steel septic tanks are prohibited.
(p) All tanks shall be clearly and permanently marked with the manufacturer's name and registered trademark, the month and year of manufacture, the maximum recommended depth of earth cover in feet or meters and the design liquid capacity of the tank. The markings shall be adequately protected from corrosion so as to remain permanent and readable over the life of the tank.
(q) A septic tank effluent filter approved by the Administrative Authority shall be installed on all new private sewage disposal systems. The filter shall prevent the passage of solids larger than $1 / 8$ ineh $\underline{1 / 8^{\prime \prime}}(3.2 \mathrm{~mm})$ in diameter while under 2 feet $\underline{2}$ ' $(0.61 \mathrm{~m})$ of hydrostatic head. The filter shall be constructed of materials that are resistant to corrosion and erosion and be of adequate size for the anticipated hydraulic and organic loading.

## †K 8 Disposal Fields

(a) Distribution lines shall be constructed of clay tile laid with open joints, perforated clay pipe, perforated high density polyethylene pipe, perforated ABS pipe, perforated PVC pipe, or other approved materials, provided that sufficient openings are available for distribution of the effluent into the trench area.
(b) Before placing aggregate or drain lines in a prepared excavation, all smeared or compacted surfaces shall be removed from trenches by raking to a depth of 1 ineh $\underline{1 "}(2.5 \mathrm{~cm})$ and the loose material removed. Aggregate shall be placed in the trench to the depth and grade required by this Section. Drainpipe shall be placed on aggregate in an approved manner. The drain lines shall than be covered with aggregate to the minimum depth required by this section and this covered with landscape filter fabric, geotextile, or similar porous material to prevent closure of voids with earth backfill. No earth backfill shall be placed over the aggregate cover until after inspection and acceptance.
Exception:
Listed or approved leaching chambers may be used in lieu of pipe and aggregate. Chamber installations shall follow this appendix for disposal fields, where applicable, and shall conform to manufacturer's installation instructions.
(c) A grade board staked in the trench to the depth of aggregate shall be utilized when distribution line is constructed with drain tile or a flexible pipe material which will not maintain alignment without continuous support.
(d) Where two or more drain lines are installed, an approved distribution box of sufficient size to receive all lateral lines and flows shall be installed at the head of each disposal field. The inverts of all outlets shall be level and the invert of the inlet shall be at least 1 ineh $\underline{1 "}(2.5 \mathrm{~cm})$ above the outlets. Distribution boxes shall be designed to ensure equal flow and shall be installed on a stable level surface such as a concrete slab or natural or compacted soil. Concrete distribution boxes shall be protected from corrosion by coating with an appropriate bituminous coating, or constructed of concrete with a $15 \%$ to $18 \%$ fly ash content, or by other approved methods acceptable to the Administrative Authority.

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(e) All laterals from a distribution box to the disposal field shall be approved pipe with watertight joints. Multiple disposal field laterals, wherever practicable, shall be of uniform length.
(f) Connections between a septic tank and a distribution box shall be laid with approved pipe with watertight joints on natural ground or compacted fill.
(g) Disposal fields and beds shall be constructed as specified in Tables $\ddagger \underline{K}-4$ (A) and $\ddagger \underline{K}-4$ (B) and the following criteria:

| Gravity Trenches | Minimum | Maximum |
| :---: | :---: | :---: |
| Number of trenches ${ }^{+}$(see footnote 1) | 1 | - |
| Length of trench | - | 100 feet 100 ' ( 30.5 m ) |
| Bottom width of trench | 12 inches $\underline{12 "}$ ( 30.5 cm ) | 3 inches $\underline{3 "}(91.4 \mathrm{~cm})$ |
| Depth of cover over disposal pipe | 9 -inches 9" $(22.9 \mathrm{~cm})$ | 24 inches $\underline{24 "}(61.0 \mathrm{~cm})^{2}$ (see footnote 2) |
| Aggregate material under disposal pipe | 12 inches $\underline{12 "}(30.5 \mathrm{~cm})$ | - |
| Aggregate material over disposal pipe | 2 inches 2" $(5.1 \mathrm{~cm})$ | $Z$ inehes 2" ( 5.1 cm ) |
| Slope of disposal pipe | Level | Level |
| Disposal pipe diameter | 3 inches $\underline{\text { 3" }}$ ( 7.6 cm ) | 4 incher ${ }^{\prime \prime}$ ( 10.1 cm ) |
| Spacing of disposal pipe, or leaching | $z$ Two $x$ effective depth ${ }^{3}$ (see footnote 3) or 5feet 5' ( 1.5 m ) whichever is greater |  |

## Notes Footnotes:

+1 Two trenches are recommended.
${ }^{z} \underline{2}$ For more than 24 inches $24^{\prime \prime}(61.0 \mathrm{~cm})$, SDR 35 or equivalent strength pipe is required.
${ }^{3} \underline{3}$ The distance between the bottom of the disposal pipe and the bottom of the trench bed.

| Gravity Beds | Minimum | Maximum |
| :---: | :---: | :---: |
| Number of disposal pipes | 2 | - |
| Length of bed | - | 100 feet $100{ }^{\prime}(30.5 \mathrm{~m})$ |
| Distance between disposal pipes | 4 feet $\underline{\prime}^{\prime}(1.2 \mathrm{~m})$ | 6 feet $\underline{6}^{\prime}(1.8 \mathrm{~m})$ |
| Width of bed | 10 feet $10^{\prime}(3.0 \mathrm{~m})$ | 12 feet $\underline{12}$ ' ( 3.66 m ) |
| Distance from pipe to sidewall | 3 feet ${ }^{\prime}$ ( 0.91 m ) | 3 feet $3^{\prime}(0.91 \mathrm{~m})$ |
| Depth of cover over disposal pipe | 9 inches 9" ( 22.9 cm ) | 14 inches $\underline{14 "}$ ( 35.6 cm ) |
| Aggregate material under disposal pipe | 12 inches $\underline{12 "}$ ( 30.5 cm ) | - |
| Aggregate material over disposal pipe | 2 incher 2" $(5.1 \mathrm{~cm})$ | Z incher 2" $(5.1 \mathrm{~cm})$ |
| Slope of disposal pipe | Level | Level |
| Disposal pipe diameter | 3 inches $\underline{\text { 3" }}$ ( 7.6 cm ) | 4 inches 4" $(10.1 \mathrm{~cm})$ |

Disposal fields, trenches and leaching beds shall not be paved over or covered by concrete or any material that can reduce or inhibit any possible evaporation of sewer effluent.
(h) When necessary on sloping ground to maintain a level disposal pipe, leach trenches or disposal beds shall be stepped. The lines between each horizontal leaching section shall be made with approved watertight joints and installed on natural or unfilled ground.
† K 9 Seepage Pits
(a) Seepage pits constructed in accordance with this appendix are considered a method of disposing of septic tank effluent. Criteria used for determining the suitability of a seepage pit are contained in Table $\ddagger \underline{K} 5$. The capacity of seepage pits shall be based on the quantity of liquid waste discharging there into, and on the character and porosity of the surrounding soil and shall conform to Section $\ddagger \underline{K} 5$ of this appendix.
(b) Multiple seepage pit installations shall be served through an approved distribution box or be connected in series by means of a watertight connection laid on undisturbed or compacted soil. The outlet from the pit shall have an approved sanitary tee with the vertical leg extending at least 12 feet $12^{\prime}(30.5 \mathrm{~cm})$ below the inlet fitting.
(c) Each seepage pit shall be circular in shape and shall have an excavated diameter of not less than 4 feet $\underline{4}^{\prime}(1.2 \mathrm{~m})$.

Approval shall be obtained prior to construction for any pit having an excavated diameter greater than 6 feet $\underline{6}$ ' $(1.8 \mathrm{~m})$.
(d) For gravel filled seepage pits, the entire pit shall be backfilled with aggregate which shall be clean and of uniform gradation, $3 / 4$ inches $\underline{3 / 4 "}(1.9 \mathrm{~cm})$ to $Z 1 / 2$ inches $21 / 2^{\prime \prime}(6.4 \mathrm{~cm})$ in diameter. Material used for backfill shall offer a minimum of $30 \%$ void space. Each pit shall have a breather/effluent conductor pipe, which shall consist of a perforated pipe at least 4 inches $\underline{" \prime}(10.2 \mathrm{~cm})$, in diameter, placed vertically within the backfill of the pit. The pipe shall extend from the bottom of

(e) Lined, hollow pits shall be lined with concrete liner, or other approved materials and shall be laid on a firm foundation. Excavation voids behind the liner shall have a minimum of 9 inches $9 "(22.9 \mathrm{~cm})$ of aggregate which shall be clean and of uniform gradation, $3 / 4$ inches $3 / 4 "(1.9 \mathrm{~cm})$ to $21 / 2$ inches $21 / 2 "(6.4 \mathrm{~cm})$ in diameter.
(f) The cover of a lined seepage pit shall be constructed of an approved 4 one or 2 two piece reinforced concrete slab of 2500 pounds per square inch ( $17,238 \mathrm{kPa}$ ) minimum compressive strength, not less than 5 inches $\underline{5 "}(127 \mathrm{~mm})$ thick and designed to support an earth load of not less than 400 pounds per square foot ( 19.2 kPa ). Each cover shall be provided with a 12 inches $12 "(30.5 \mathrm{~cm})$ minimum access hole with plug or cover and shall be coated on the underside with an approved bituminous seal or constructed of concrete with $15 \%$ to $18 \%$ fly ash content or other nonpermeable protective material. Each cover shall have at least a 4 inch $\underline{" \prime}(10.2 \mathrm{~cm})$ inspection pipe placed vertically not more than 6 inches $\underline{6^{\prime \prime}}$ below ground level.
(g) The top of the seepage pit cover must be at least 48 inehes 18 " $(45.7 \mathrm{~cm})$ but not more than 4 feet $\underline{\prime}^{\prime}(1.2 \mathrm{~m})$ below the surface of the ground.
(h) An approved vented inlet fitting shall be provided in every seepage pit to prevent the inflow from damaging the sidewall. Exception: When using a one or two piece concrete slab cover inlet, the fitting may be a $1 / 4$ bend fitting discharging through an opening in the top of the slab cover. For multiple seepage pit installations, the outlet fittings shall be per Section $\ddagger \underline{K} 9$ (b) of this appendix.
(i) Seepage pit design details are shown in Figure $\ddagger \underline{K}-1$ and $\ddagger \underline{K}-2$.

## 士K 10 Cesspools

The use of cesspools for waste disposal is prohibited.

## £ K 11 Interceptor Design Criteria for Private Sewage Disposal Systems

(a) When liquid wastes containing excessive amounts of grease, garbage, flammable wastes, sand, or other ingredients which may affect the operation of a private sewage disposal system, an interceptor for such wastes shall be installed.
(b) Installation of such interceptors shall comply with Section 1008.0 of the Uniform Plumbing Code and their location shall be in accordance with Table $\ddagger \underline{K}-1$ of this appendix.
(c) Sampling box shall be installed when required by the Administrative Authority.
(d) Interceptors shall be of approved design and be of not less than two compartments. Structural requirements shall be in compliance with the applicable subparts of Section $\ddagger \underline{K} 7$ of this appendix.
(e) Interceptors shall be located as close to the source as possible and be accessible for servicing. All necessary manholes for servicing shall be at grade level and be gas-tight.
(f) Waste discharge from interceptors may be connected to a septic tank or other primary system or be disposed into a separate disposal system.
(g) Recommended Design Criteria. Minimum design criteria for grease and garbage, commercial kitchens; sand-silt oil, auto washers; and silt-lint grease, laundries, and laundromats. (Formulae may be adapted to other types of occupancies with similar wastes or as determined by the Administrative Authority rules.)

| Grease and Garbage, Commercial Kitchens |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Meals | Waste |  | Retention |  | Storage |  | Interceptor Size |
| per peak hour | x | Flow Rate | x | Time | x | Factor | $=$ |
| (liquid capacity) |  |  |  |  |  |  |  |


| Sand-Silt Oil, Auto Washers |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of Vehicles |  |  |  |  |  |  |  |
| per peak hour | x | Waste |  | Retention | Storage |  | Interceptor Size <br> (liquid capacity) |


| Silt-Lint Grease, Laundries, Laundromats |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Number of |  | ZTwo cycles |  | Waste |  | Retention |  | Storage |  | Interceptor Size |
| Machines | x | per hour | x | Flow Rate | x | Time | X | Factor | $=$ | (liquid capacity) |

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## Waste Flow Rate

See Table $\ddagger \mathrm{K}-3$ of this appendix for estimated flow rates.

## Estimated Retention Times

| Commercial kitchen waste: <br> Dishwasher and/or disposal | 2.5 hours |
| :--- | :---: |
| Single Service kitchen: <br> Single serving with disposal | 1.5 hours |
| Sand-silt-oil | 2.0 hours |
| Lint-silt (laundry) | 2.0 hours |

## Estimated Storage Factors

| Fully equipped commercial kitchen | 8 hour operation: | 1.0 |
| :--- | :---: | :---: |
|  | 16 hour operation: | 2.0 |
|  | 24 hour operation | 3.0 |
| Single service kitchen |  | 1.5 |
| Auto washers | Self-serve | 1.5 |
|  | Employee operated | 2.0 |
| Laundries, Laundromats | (allows for rock filter) | 1.5 |

## † K 12 Inspection and Testing

(a) Private sewage disposal systems shall be inspected and tested prior to operation.
(b) Inspection shall be for the following purposes:
(1) To verify soil characteristics used for the basis of the design.
(2) To verify the installation of approved equipment and materials.
(3) To verify that construction was performed in accordance with the permit.
(4) To verify watertightness of the septic tank and other components.
(c) Field testing shall include:
(1) Septic tank watertightness - Watertightness shall be established before inspection. A tank failing the watertightness test must be repaired or replaced, and cannot be operated until it complies with watertightness requirements and has been inspected.
(2) Water test procedures - Tanks shall be filled to the invert of the outlet. Water shall be left standing in the tank for at least 24 hours before the inspection. After 24 hours, refill the tank, if necessary. Concrete may absorb some water. At the start of the inspection, record the initial water level and time. After one hour, record the time and the corresponding water level. A tank shall pass a watertightness test if the water level dropped less than $1 / 4$ of an inch. A visible leak (flowing water) shall be considered a failure. A damp or wet spot that is not flowing is not considered a failure.
(3) Vacuum testing procedures - Vacuum testing may be used to determine watertightness in lieu of a water test. The tank shall be sealed and empty. A vacuum of 2 inehes $\underline{2 "}(5.1 \mathrm{~cm})$ of mercury ( 1 psi or $69.1 \mathrm{gm} / \mathrm{sq}$. cm ) shall be applied and stabilized. The vacuum shall drop no more than 0.2 inches $0.2^{\prime \prime}(0.5 \mathrm{~cm})$ of mercury ( 0.1 psi or $6.9 \mathrm{gm} / \mathrm{sq}$. cm $)$ during the one hour test period.
(4) A flow test shall be performed through the system to the point of effluent disposal. All lines and components shall be watertight. Capacities, required air space, and fittings shall be in accordance with the provisions set forth in this appendix.

## ŁK 13 Abandoned Sewers and Sewage Disposal Facilities

(a) Every abandoned building (house) sewer, or part thereof, shall be plugged or capped in an approved manner within five feet 5 ' $(1.5 \mathrm{~m})$ of the property line.
(b) Every cesspool, septic tank, alternative sewage disposal system, and seepage pit which has been abandoned or has been discontinued otherwise from further use or to which no waste or soil pipe from a plumbing fixture is connected, shall have the sewage removed therefrom, shall have all electrical and mechanical components disconnected and abandoned under
the appropriate procedures in the uniform building or electrical code, as applicable, and be completely filled with earth, sand, gravel, concrete, or other approved material.
(c) The top cover or arch over the cesspool, septic tank, or seepage pit shall be removed before filling and the filling shall not extend above the top of the vertical portions of the sidewalls or above the level of any outlet pipe until inspection has been called and the cesspool, septic tank, or seepage pit has been inspected. After such inspection, the cesspool, septic tank, or seepage pit shall be filled to the level of the top of the ground.
(d) No person owning or controlling any cesspool, septic tank, or seepage pit on the premises of such person or in that portion of any public street, alley, or other public property abutting such premises, shall fail, refuse, or neglect to comply with the provisions of this section or upon receipt of notice so to comply from the Administrative Authority.
(e) Where disposal facilities are abandoned consequent to connecting any premises with the public sewer, the permittee making the connection shall fill all abandoned facilities as required by the Administrative Authority within 30 days from the time of connecting to the public sewer.

## £ K 14 Drawings and Specifications

The Administrative Authority may require any or all of the following information before a permit is issued for a private sewage disposal system:
(1) A site specific plot plan drawn to scale, dimensioned, showing direction and approximate slope of surface ( 2 foot $\underline{2}$ ' ( 0.6 $\mathrm{m})$ contour lines), location of all present or proposed retaining walls, drainage channels, water supply lines or wells, paved areas and structures on the plot, number of bedrooms or plumbing fixtures in each structure and location of the private sewage disposal system with relation to lot lines and structures.
(2) Details of construction including system profile and construction sections necessary to assure compliance with the requirements of this appendix together with a full description of the complete installation including specifications describing all materials, equipment, construction, workmanship, and methods of assembly and installation.
(3) A $\log$ of the soil formations, percentage of rock, texture, structure, consistence, and mottles as provided in ASTM D-5921-96, or other practice acceptable, and depth to the groundwater below the land surface as determined by established records or test holes dug in close proximity to any proposed seepage pit or disposal field, together with a statement of water absorption characteristics of the soil at the proposed site as determined by site investigation and soil evaluation.
†K 15 Percolation Testing, Soil Absorption Rate and Minimum vertical separation
(a) The following procedures are to be used for determining the system soil absorption rate and minimum vertical separation:
(1) Establish the primary area based on site suitability review including proposed improvements.
(2) Establish the reserve area based on site suitability review and the type of system.
(3) Excavate the test hole to the depth necessary to confirm soil conditions for the design of the proposed disposal trench, bed or seepage pit. For disposal trenches and beds, a minimum of one test hole at both the primary disposal area and the reserve area is required. For seepage pits, one test hole is required at the primary disposal area.
(4) For disposal trenches and beds, testing shall be performed at appropriate locations and depths within the soil profile to determine the rate at which the soil will absorb effluent. Percolation tests should be performed at each obvious strata change that would significantly effect the design, soil application rate or minimum vertical separation.
(5) For seepage pits, discount the thickness of any relatively impermeable soil profiles while determining the soil absorption rate.
(6) A test hole shall not be excavated within 3 feet $3^{\prime}(0.9 \mathrm{~m})$ of a ledge in an observation pit. Care should be taken to assure adjacent features impacting the absorption rate are avoided.
(7) The test report shall include a site evaluation map locating the test hole(s).
(b) Disposal Trenches and Beds
(1) Area Preparation
(i) Dig a 12 ineh $\underline{12 "}$ square by 12 ineh $\underline{12 " \prime}(30.5 \mathrm{~cm} \times 30.5 \mathrm{~cm})$ deep hole or a 15 inch $\underline{15 "}$ round by 12 ineh $\underline{12 "}$ ( $38.1 \mathrm{~cm} \times 30.5 \mathrm{~cm}$ ) deep hole in undisturbed soil.
(ii) Scarify any smeared soil surfaces.
(iii) Remove loosened materials from the bottom of hole.
(2) Presoaking the Test Hole
(i) A system, such as a perforated bucket, can be used to support the sidewalls of the test hole if necessary. Fill any voids between the walls of the hole and the bucket with pea gravel.
(ii) Fill the test hole to 12 inches $\underline{12 "}(30.5 \mathrm{~m})$ above the bottom of the hole with clean water.
(iii) Observe the rate at which the water level drops in the hole and determine the time in minutes for the water to completely drain away.
(iv) If the water drains away in less than 60 minutes, repeat the procedure. If the water drains away in less than an additional 60 minutes, repeat the procedure a third time.
(v) Proceed immediately with testing if the water drains away three times in less than 60 minutes each time.
(vi) If the water does not drain after the third refill, maintain the water level in the test hole at a minimum depth of 9

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inches $\underline{9 "}(22.9 \mathrm{~cm})$ for at least four hours. Wait for a minimum of 16 hours and a maximum of 24 hours before proceeding with the test.
(3) Conducting the Test
(i) Remove any materials that have sloughed into the test hole to be sure that the test hole has the dimensions indicated above.
(ii) Fill the hole with clean water to a depth of 6 inches 6 " $(15.2 \mathrm{~cm})$ above the bottom of the percolation test hole.
(iii) Measure the time it takes for the water level to drop exactly 1 ineh $\underline{1 "}(2.5 \mathrm{~cm})$ from a fixed reference point. Record the drop in the water level. Care should be used to be sure that the measurement method does not have a significant impact on determination of the absorption rate.
(iv) Determine the stabilized absorption rate for the test hole. The approximate absorption rate can be determined by repetitively measuring the absorption rate until three consecutive values vary by no more than $10 \%$. If three consecutive measurements indicate that the absorption rate is not approaching a steady rate or that the rate is close to a restrictive limit, an alternate method based on a graphical solution of the test data approximating the final stabilized rate is recommended.
(v) Record the test rate based on the above procedure.
(c) Seepage Pits
(1) Area Preparation
(i) Drill a test hole at least 48 inehes $\underline{18 "}(45.7 \mathrm{~cm})$ in diameter to the depth of the bottom of the proposed seepage pit. The minimum hole depth is 30 feet $30^{\prime}(9.1 \mathrm{~m})$. After the test, the diameter of the test hole may be enlarged to allow construction of the seepage pit at the same location.
(ii) Scarify any smeared soil surfaces.
(iii) Remove loosened materials from the bottom of the hole.
(2) Presoaking the Test Hole.
(i) Fill the bottom 6 inehes $\underline{6}^{\prime \prime}(15.2 \mathrm{~cm})$ of the test hole with gravel, if necessary, to prevent scouring.
(ii) Fill the test hole with clean water up to 3 feet $\underline{3}^{\prime}(0.9 \mathrm{~m})$ below grade.
(iii) Observe the rate at which the water level drops in the hole and determine the time in minutes for the water to completely drain away.
(iv) If the water drains away in less than four hours, repeat the procedure.
(v) Proceed immediately with testing if the water drains away in less than four hours after the second refill.
(vi) If the water does not drain within four hours after the second refill, refill the hole a third time and wait for a minimum of 16 hours and a maximum of 24 hours before proceeding with the test.
(vii) If there is still standing water in the hole after the presoaking has been completed, the water shall not be removed from the hole.
(3) Conducting the Test
(i) Fill the hole with clean water up to 3 feet $3^{\prime}(0.9 \mathrm{~m})$ below grade.
(ii) Measure the decline of the water level from a fixed reference point every 10 minutes until a stabilized absorption rate is obtained.
(iii) Determine the stabilized absorption rate by repetitively measuring the absorption rate until three consecutive values vary by no more than $10 \%$. If three consecutive measurements indicate that the absorption rate is not approaching a steady rate or that the rate is close to a restrictive limit, an alternate method based on a graphical solution of the test data approximating the final stabilized rate is recommended.
(iv) Do not use test results from any relatively impermeable soil profiles while determining the stabilized soil absorption rate.
(v) Record the rate based on the above procedure.

Table $\ddagger \underline{K}-1$ Location of Sewage Disposal System

| Point of Beginning for Setback Distance | Minimum Horizontal Setback Distances in feet (meters) |  |
| :---: | :---: | :---: |
|  | Septic Tank | Disposal Trench/Bed or Seepage Pit |
| Buildings ${ }^{\text { }}$ (see footnote 1) | 10 (3.0) | 10 (3.0) |
| Property line adjoining private property | 5 (1.5) | (See footnote 2) |
| Well (Public Water Supplies) | 100 (30.5) | 100 (30.5) |
| Wells (Private) ${ }^{3}$ (see footnote 3) | 100 (30.5) | 100 (30.5) |
| Live Streams ${ }^{4}$ (see footnote 4) | 100 (30.5) | 100 (30.5) |
| Lake or Reservoir ${ }^{5}$ (see footnote 5) | 100 (30.5) | 100 (30.5) |
| Domestic Water Source Intake | 200 (61.0) | 200 (61.0) |
| Dry Wash/Drainage Easement ${ }^{6}$ (see footnote 6) | 50 (15.2) | 50 (15.2) |
| Transmission Distribution Water Line | 10 (3.0) | 10 (3.0) |
| Domestic service Water Line ${ }^{7}$ (see footnote 7) | 5 (1.5) | 5 (1.5) |
| Cut on Sloping downgradient Terrain, Culverts and Roadway Ditches ${ }^{8}$ (see footnote 8) | 15 (4.6) | 15 (4.6) or 4-four $x$ the elevation difference between the finished grade at the point of beginning and the elevation at the cut bank bottom, ditch bottom, or culvert invert, whichever is greater, up to 50 feet 50 ' $(15.2 \mathrm{~m})$ |
| Driveway ${ }^{9}$ (see footnote 9) | 5 (1.5) | 5 (1.5) |
| Swimming Pool ${ }^{10}$ (see footnote 10) | 5 (1.5) | 5 (1.5) |
| Any Easements (other than drainage easements) ${ }^{4}$ (see footnote 11) | 5 (1.5) | 5 (1.5) |

Notes Footnotes:
${ }^{+} 1$ Including porches, decks and steps, whether covered or uncovered, breezeways, roofed patios, carports, covered walks, covered driveways, swimming pools, and similar structures and appurtenances.
${ }^{z_{2}}$ The setback requirement is five feet $5^{\prime}(1.5 \mathrm{~m})$ unless the property is not served by a central system for the distribution of water and:
(a) There is no existing or proposed individual well on adjoining private property, in which case the setback is 50 feet 50 ' $(15.2 \mathrm{~m})$; or
(b) A 100 foot 100 ' $(30.5 \mathrm{~m})$ separation distance cannot be maintained from an existing or proposed individual well, in which case the setback is the distance necessary to maintain the 100 foot $100^{\prime}(30.5 \mathrm{~m})$ separation; or
(c) The applicable setback requirement in (a) or (b) may be reduced to a minimum of five feet $5^{\prime}(1.5 \mathrm{~m})$ with a variance from the Administrative Authority.
${ }_{4}^{3} \frac{3}{4}$ For unaltered lots in a subdivision approved before October 1, 1986.
${ }_{5}^{4}$ Measured from the nearest boundary of peak streamflow from a 10 year, 24 hour precipitation event.
${ }^{5} 5$ Measured from the elevation of high water line from a peak flow from a 10 year, 24 hour precipitation event at the spillway.
${ }^{6} \underline{6} \quad 50$ Fifty foot $(15.2 \mathrm{~m})$ setback is measured from the edge of the defined natural channel bank of a drainage area of more than 5 five acres or a drainage easement whichever is less. Setback may be reduced to 25 feet $25^{\prime}$ ( 7.6 m ) up gradient from the system, if channel erosion protection is provided (naturally or man-made) and approved by the Administrative Authority.
${ }^{7}$ Z Water pipes crossing or adjacent to sewer or drainage piping constructed of clay or materials that are not approved for use within a building shall be laid a minimum of 12 inehes $\underline{12 "}(30.5 \mathrm{~cm})$ from the sewer or drain pipe.
${ }^{8} \underline{8}$ Measure the setback from the outside of the private sewage disposal system component to the top of the cut bank or ditch, or to the nearest sidewall of a culvert.
${ }^{9} \underline{9}$ Measured from the edge of the driveway to the nearest edge of septic tank excavation. A properly reinforced septic tank and cover may be placed at any location relative to a driveway if access openings, risers, and covers carry the design load and are protected from inflow.

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${ }^{10} 10 \quad$ Setback may be increased due to soil loading and stability concerns.
${ }^{41} \underline{11} \quad 5$ Five feet $(1.5 \mathrm{~m})$ minimum unless other setback requirements govern.
Table $\ddagger \underline{K}-2$ Design liquid capacity (size) of septic tanks

| No. of Bedrooms | No. of Occu- <br> pants | No. of <br> Baths | Maximum Fix- <br> ture Count | Recommended Sep- <br> tic Tank Size in <br> Gallons (L) | Minimum Septic <br> Tank Size in Gallons <br> $(\mathbf{L})(+$ or 5\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 2 | 4 | 1 | 12 | $1000(3785)$ | $1000(3785)$ |
| 2 | 4 | 2 | 18 | $1000(3785)$ | $1000(3785)$ |
| 3 | 6 | 1 | 18 | $1250(4731)$ | $1000(3785)$ |
| 3 | 6 | 2 | 18 | $1250(4731)$ | $1000(3785)$ |
| 4 | 8 | 2 | 24 | $1500(5678)$ | $1250(4731)$ |
| 4 | 8 | 3 | 25 | $1500(5678)$ | $1250(4731)$ |
| 5 | 10 | 2 | 30 | $2000(7570)$ | $1500(5678)$ |
| 5 | 10 | 3 | 30 | $2000(7570)$ | $1500(5678)$ |
| 5 | 10 | 4 | 32 | $2000(7570)$ | $1500(5678)$ |
| 6 | 12 | 3 | 36 | $2500(9463)$ | $2000(7570)$ |
| 6 | 12 | 4 | 36 | $2500(9463)$ | $2000(7570)$ |
| 6 | 12 | 5 | 39 | $2500(9463)$ | $2000(7570)$ |
| 7 | 14 | 3 | 42 | $2500(9463)$ | $2000(7570)$ |
| 7 | 14 | 4 | 42 | $2500(9463)$ | $2000(7570)$ |
| 7 | 14 | 5 | 42 | $2500(9463)$ | $2000(7570)$ |

Table $\ddagger \underline{K}-3$ Estimated Waste/Sewage Flow Rates
Because of the many variables encountered, it is not possible to set absolute values for waste/sewage flow rates for all situations. The designer should evaluate each situation and, if figures in this table need modification, they should be made with the concurrence of the Administrative Authority.

| Type of Occupancy | Gallons/Liters Per Day |
| :--- | :---: |
| 1. Airports | $15(56.7)$ per employee <br> $5(18.9)$ per passenger |
| 2. Auto Washers | Per manufacturer's specification |
| 3. Bowling Alleys (snack bar only) | $75(283.9)$ per lane |
| 4. Camps |  |
| Campground with central comfort station | $35(132.4)$ per person |
| With flush toilets, no showers |  |
| Day camps (no meals served) | $25(94.6)$ per person |
| Summer and seasonal | $15(56.7)$ per person |
| 5. Churches (Sanctuary) | $50(189.2)$ per person |
| With kitchen waste | $5(18.9)$ per seat |
| 6. Dance halls | $7(26.4)$ per seat |
| 7. Factories | $5(18.9)$ per person |
| No showers | $25(94.6)$ per employee |
| With showers | $35(132.4)$ per employee |
| Cafeteria, add | $5(18.9)$ per employee |


| 8. Hospitals Kitchen waste only Laundry waste only | 250 (946.3) per bed 25 (94.6) per bed 40 (151.4) per bed |
| :---: | :---: |
| 9. Hotels With kitchen Without kitchen | 60 (227.1) per bed (2 person) 50 (189.2) per bed (2 person) |
| 10. Institutions (Resident) | 75 (283.9) per person |
| Nursing home Rest home | 125 (473.1) per person 125 (473.1) per person |
| 11. Laundries, self service (minimum 10 hours per day) Commercial | 50 (189.2) per wash cycle Per manufacturer's specification |
| 12. Motel With kitchen Without kitchen | 60 (227.1) per bed (2 person) 50 (189.2) per bed ( 2 person) |
| 13. Offices | 20 (75.7) per employee |
| 14. Parks <br> Mobile homes <br> Picnic parks (toilets only) <br> Recreational vehicles <br> Without water or sewer hook-up With water and sewer hook-up | 250 (946.3) per parking space 20 (75.7) per parking space <br> 75 (283.9) per parking space 100 (378.5) per parking space |
| 15. Restaurants cafeterias <br> Toilet <br> Kitchen waste <br> Garbage disposal <br> Cocktail lounge <br> Kitchen waste <br> Disposal service | 20 (75.7) per employee <br> 7 (26.4) per customer 6 (22.7) per meal 1 (3.7) per meal 2 (7.5) per customer <br> 2 (7.5) per meal |
| 16. Schools- Staff and office <br> Elementary <br> Middle and high With gym and showers, add With cafeteria, add Boarding, total waste | 20 (75.7) per person 15 (56.7) per student 20 (75.7) per student 5 (18.9) per student 3 (11.3) per student 100 (378.5) per person |
| 17. Service station, toilets | 1000 (3785.4) for 1st bay 500 (1892.7) for each additional bay |
| 18. Stores Public restrooms, add | $\begin{gathered} 20(75.7) \text { per employee } \\ 1 \text { per } 10 \text { sq. ft. of floor space } \\ (3.79 \text { per } 9290.3 \text { sq. } \mathrm{cm} \text { of floor space) } \end{gathered}$ |
| 19. Swimming pools, public | $10(37.8)$ per person |
| 20. Theaters, auditoriums Indoor Drive-in | $\begin{gathered} 5(18.9) \text { per seat } \\ 10(37.8) \text { per space } \end{gathered}$ |
| 21. Single Family Residential Dwellings | $150(567.7)$ per bedroom or $25(94.6)$ per fixture unit whichever is greater |

22. For structures and facilities not specifically addressed in the above table, flow rates available from other standard books and literature may be approved.
(a) Recommended Design Criteria. The size of the sewage disposal system components, for uses other than a single family

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residential dwelling, is calculated as follows:
For waste/sewage flow, up to 3000 gallons/day (11,355 liters/ day)
Design Flow $=$ The total of the estimated flow rates from Table $\ddagger \underline{K}-3$
Septic tank size $=$ design flow $\times 2.1$.
(b) Also see Section $\ddagger \underline{K} 3$ of this appendix.
(c) Additional treatment of sewage is required if sewage quality exceeds 430 milligrams/liter ( 7,362 grans per gallon) for total suspended solids, 380 milligrams/liter ( 6,506 grans per gallon) for the five-_day biochemical oxygen demand, 75 milligrams/liter ( 1,284 grans per gallon) for fats, oils, and greases, or the sewage includes wastes other than those originating from domestic toilet flushing, food preparation, non-occupational laundry, or personal hygiene, or wastes originating from an operation using any hazardous substance or creating a hazardous waste as defined in the statutes or rules of the Arizona Department of Environmental Quality.
Table $\mp$ K-4 (A) Soil Absorption Rate and Minimum Vertical Separation for Shallow Disposal Field Systems by Soil Evaluation Method ${ }^{+}$(see footnote 1)
Instructions: Read questions in the following table beginning with row A. The first "yes" response from columns arb $\underline{A}$ or B determines the maximum soil absorption rate.

|  | A <br> Soil Absorption Rate in gallons per day (gpd) $\left[\begin{array}{c} \text { per sq. ft. } \\ {\left[\begin{array}{c} \text { liters per day } \\ \text { (lpd) sq. } m \end{array}\right]} \end{array}\right.$ | B <br> Soil Absorption Rate in sq. ft. per 100 gallons $\left[\begin{array}{c} \text { per day (gpd) } \\ {\left[\begin{array}{c} \text { sq. } \mathrm{m} \cdot \text { per } 100 \text { liters per } \\ \text { day (lpd) } \end{array}\right]} \end{array}\right.$ |
| :---: | :---: | :---: |
| A. Is the horizon gravelly coarse sand or coarser? | 0 (0) | 0 (0) |
| B. Is the structure of the horizon moderate or strongly platy? | 0 (0) | 0 (0) |
| C. Is the texture of the horizon sandy clay loam, clay loam, silty clay loam, or finer and structure weak platy? | 0 (0) | 0 (0) |
| D. Is the moist consistence stronger than firm or any cemented class? | 0 (0) | 0 (0) |
| E. Is texture sandy clay, clay, or silty clay of high clay content and structure massive or weak? | 0 (0) | 0 (0) |
| F. Is texture sandy clay loam, clay loam, silty clay loam, or silty loam and structure massive? | 0 (0) | 0 (0) |
| G. Is the texture of the horizon loam or sandy loam and the soil structure massive? | . 20 (8.15) | 500 (12.3) |
| H. Is texture sandy clay, clay, or silty clay of low clay content and the structure moderate or strong? | . 20 (8.15) | 500 (12.3) |
| I. Is texture sandy clay loam, clay loam, or silty clay loam and structure weak? | . 20 (8.15) | 500 (12.3) |
| J. Is texture sandy clay loam, clay loam, or silty clay loam and structure moderate or strong? | . 40 (16.30) | 250 (6.1) |
| K. Is texture sandy loam, loam, or silty loam and structure weak? | . 40 (16.30) | 250 (6.1) |
| L. Is texture sandy loam, loam, silt loam and structure moderate or strong? | . 60 (24.45) | 166.7 (4.1) |
| M. Is texture fine sand, very fine sand, loamy fine sand, or loamy very fine sand? | . 40 (16.30) | 250 (6.1) |
| N. Is texture loamy sand or sand? | . 80 (32.59) | 125 (3.1) |
| O. Is texture coarse sand? ${ }^{2}$ (see footnote 2) | 1.20 (48.89) | 83.3 (2.0) |

Notes Footnotes:
${ }^{4} 1$ Shallow trench and bed systems are less than 5 feet 5 ' $(1.52 \mathrm{~m})$ deep and shall have a minimum vertical separation of 4 feet $\underline{4}^{\prime}(1.22 \mathrm{~m})$ to rock, fractured rock, soils with greater than $50 \%$ rock fragments and a 5 foet $\underline{5}^{\prime}(1.52 \mathrm{~m})$ zone of unsaturated soil to groundwater.
${ }^{z} \underline{2}$ Shallow trench and bed systems are less than 5 feet $5^{\prime}(1.52 \mathrm{~m})$ deep and shall have a minimum vertical separation of 10 feet $\underline{10^{\prime}}(3.04 \mathrm{~m})$ to rock, fractured rock, soils with greater than $50 \%$ rock fragments and a 10 feet $\underline{10^{\prime}}(3.04 \mathrm{~m})$ zone of unsaturated soil to groundwater.
Example:
Three-bedroom dwelling, wastewater flow rate $=450 \mathrm{gpd}(1,703 \mathrm{lpd})$, soil texture is loamy sand $(\mathrm{N})$.
Using Column A - Effective Area = Divide the flow rate ( 450 gpd ) ( $1,703 \mathrm{lpd}$ ) by the soil application rate of $0.8 \mathrm{gpd} /$ sq. ft ( $32.6 \mathrm{lpd} /$ sq. m.)
Effective Area $=450 / 0.8=562.5$ sq. $\mathrm{ft}(1,703 / 32.6=52.3$ sq. m. $)$.
Using Column B - Effective Area = Multiply the flow rate (450 gpd) (1,703 lpd) by the soil application rate of 125 sq. ft./100 gpd ( $3.1 \mathrm{sq} . \mathrm{m} . / 100 \mathrm{lpd}$ )
Effective Area $=450 \times 125 / 100=562.5$ sq. ft $(3.1 \times 1,703 / 100=52.3$ sq. m. $)$.
Table $\ddagger \underline{\mathbf{K}} \mathbf{- 4}$ (B) Soil Absorption Rate and Minimum Vertical Separation for Deep Disposal Field Systems by Soil Evaluation Method ${ }^{+1}$ (see footnote)
Instructions: Read questions in the following table beginning with row A. The first "yes" response from columns A or B determines the maximum soil absorption rate.

|  | A <br> Soil Absorption Rate in gallons per day (gpd) $\left[\begin{array}{c} \text { per sq. } \mathrm{ft} . \\ \text { liters per day } \\ \text { (lpd) sq. } \mathrm{m} \end{array}\right]$ | B <br> Soil Absorption Rate in sq. ft. per 100 gallons $\left[\begin{array}{c} \text { per day (gpd) } \\ {\left[\begin{array}{c} \text { sq. m. per } 100 \text { liters per } \\ \text { day (lpd) } \end{array}\right]} \end{array}\right.$ |
| :---: | :---: | :---: |
| A. Is the horizon gravelly coarse sand or coarser? | 0 (0) | 0 (0) |
| B. Is the structure of the horizon moderate or strongly platy? | 0 (0) | 0 (0) |
| C. Is the texture of the horizon sandy clay loam, clay loam, silty clay loam, or finer and structure weak platy? | 0 (0) | 0 (0) |
| D. Is the moist consistence stronger than firm or any cemented class? | 0 (0) | 0 (0) |
| E. Is texture sandy clay, clay, or silty clay of high clay content and structure massive or weak? | 0 (0) | 0 (0) |
| F. Is texture sandy clay loam, clay loam, silty clay loam, or silty loam and structure massive? | 0 (0) | 0 (0) |
| G. Is the texture of the horizon loam or sandy loam and the soil structure massive? | . 13 (5.3) | 769 (18.9) |
| H. Is texture sandy clay, clay, or silty clay of low clay content and the structure moderate or strong? | . 13 (5.3) | 769 (18.9) |
| I. Is texture sandy clay loam, clay loam, or silty clay loam and structure weak? | . 13 (5.3) | 769 (18.9) |
| J. Is texture sandy clay loam, clay loam, or silty clay loam and structure moderate or strong? | . 27 (11.0) | 370.4 (9.1) |
| K. Is texture sandy loam, loam, or silty loam and structure weak? | . 27 (11.0) | 370.4 (9.1) |
| L. Is texture sandy loam, loam, silt loam and structure moderate or strong? | . 40 (16.3) | 250 (6.1) |
| M. Is texture fine sand, very fine sand, loamy fine sand, or loamy very fine sand? | . 27 (11.0) | 370.4 (9.1) |

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| N. Is texture loamy sand or sand? | $.53(21.6)$ | $188.7(4.6)$ |
| :--- | :---: | :---: |
| O. Is texture coarse sand? | $0(0)$ | $0(0)$ |

Notes Footnote:

+ Deep trench systems are greater than or equal to 5 feet $5^{\prime}(1.52 \mathrm{~m})$ and less than 10 feet $\underline{10}$, $(3.0 \mathrm{~m})$ deep and shall have a minimum vertical separation of 5 feet $\underline{5}^{\prime}(1.52 \mathrm{~m})$ to rock, fractured rock, soils with greater than $50 \%$ rock fragments and a 5 feet $\underline{5}$, $(1.52 \mathrm{~m})$ zone of unsaturated soil to groundwater.
Example:
Three bedroom dwelling, wastewater flow rate $=450$ gpd ( $1,703 \mathrm{lpd}$ ), soil texture is loamy sand $(\mathrm{N})$.
Using Column A - Effective Area = Divide the flow rate ( 450 gpd ) ( $1,703 \mathrm{lpd}$ ) by the soil application rate of $0.53 \mathrm{gpd} / \mathrm{sq}$.
$\mathrm{ft}(21.6 \mathrm{lpd})$
Effective Area $=450 / 0.53=849$ sq. ft. $(1,703 / 21.6=78.8$ sq. m. $)$.
Using Column B - Effective Area = Multiply the flow rate ( 450 gpd ) ( $1,703 \mathrm{lpd}$ ) by the soil application rate of 188.7 sq . ft./ 100 gpd ( 4.6 sq. m./100 lpd)
Effective Area $=450 \times 188.7 / 100=849$ sq. ft. $(4.6 \times 1,703 / 100)=78.3$ sq. m. $)$.
Table $\ddagger \underline{K}-5$ Soil Absorption Rate and Minimum Vertical Separation for Seepage Pits

| Soil Characteristics | Percolation Test Rate Range | Maximum Soil Absorption Rate | Minimum Vertical Separation to Groundwater |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  |  | Gravel Seepage Pit | $\begin{gathered} \text { Lined } \\ \text { Seepage Pit } \\ \hline \end{gathered}$ |
|  | minutes per inch (minutes per cm ) | gpd/sq. ft. (lpd/sq. m.) | ft (m) | ft (m) |
| Gravelly Coarse Sands | $\begin{aligned} & \text { less than } \\ & 1.00(0.39) \end{aligned}$ | greater than 1.20 (48.9) <br> NOT PERMITTED | - | - |
| Coarse to Medium to Fine Sands | $\begin{aligned} & 1.00 \text { to less than } \\ & 2.00 \\ & (0.39 \text { to less than } \\ & 0.78) \\ & \hline \end{aligned}$ | 1.20 (48.9) | 60 (18.3) | 60 (18.3) |
| Medium to Fine Sands and Silty Sands | 2.00 to less than 3.00 ( 0.78 to less than 1.18) <br> 3.00 to less than 4.00 ( 1.18 to less than 1.57) <br> 4.00 to less than 5.00 ( 1.57 to less than 1.96) 5.00 to less than 6.00 ( 1.96 to less than 2.75) | $\begin{array}{\|l\|} \hline 1.10(44.8) \\ 1.00(40.7) \\ 0.90(36.7) \\ 0.75(30.6) \end{array}$ | $\begin{aligned} & \hline 60(18.3) \\ & 60(18.3) \\ & 60(18.3) \\ & 60(18.3) \end{aligned}$ | $\begin{aligned} & \hline 60(18.3) \\ & 60(18.3) \\ & 60(18.3) \\ & 60(18.3) \end{aligned}$ |
| Fine to Very Fine sands and Loamy Sands | 7.00 to less than 10.00 (2.75 to less than 3.93) | 0.63 (25.7) | 25 (7.6) | 25 (7.6) |
| Sandy to Silty Loams, loam | 10.00 to less than 15.00 (25.4 to less than 38.1) <br> 15.00 to less than 20.00 (38.1 to less than 50.8) <br> 20.00 to less than 25.00 ( 50.8 to less than 63.5) <br> 25.00 to less than 30.00 (63.5 to less than 76.2) | $\begin{aligned} & \hline 0.50(20.4) \\ & 0.44(17.9) \\ & 0.40(16.3) \\ & 0.36(14.7) \end{aligned}$ | $\begin{aligned} & 25(7.6) \\ & 25(7.6) \\ & 25(7.6) \\ & 25(7.6) \end{aligned}$ | $\begin{aligned} & 25(7.6) \\ & 25(7.6) \\ & 25(7.6) \\ & 25(7.6) \end{aligned}$ |
| Strongly Structured Loams and Clayey Soils | greater than $30.00 \text { (76.2) }$ | less than .36 <br> NOT PERMITTED | - | - |

(a) Seepage pits shall be bored 5 feet $\underline{5}^{\prime}(1.5 \mathrm{~m})$ deeper than the proposed pit depth to verify underlying soil characteristics,
unless water table evidence or bedrock is encountered sooner. The 5 feet 5 , $(1.5 \mathrm{~m})$ of overdrill shall then be backfilled with low permeability drill cuttings or other suitable material.
(b) Seepage pits that terminate in gravelly, coarse sand stratas shall be backfilled 5 feet $\underline{5^{\prime}}(1.5 \mathrm{~m})$ above the beginning of such strata with low permeability drill cuttings or other suitable material.
Table $\ddagger$ K-6 Seepage Pit Effective Absorption Area

| Effective Strata Depth Below <br> Flow Line | Seepage Pit Effective <br> Absorption Area, sq. ft. (sq. m.) |  |  |
| :---: | :---: | :---: | :---: |
|  | Diameter of Seepage Pit |  |  |
| ft (m) | $4 \mathrm{ft}(1.2 \mathrm{~m})$ | $5 \mathrm{ft}(1.5 \mathrm{~m})$ | $6 \mathrm{ft}(1.8 \mathrm{~m})$ |
| $1(0.3)$ | $13(1.2)$ | $16(1.5)$ | $19(1.8)$ |
| $2(0.6)$ | $25(2.3)$ | $31(2.9)$ | $38(3.5)$ |
| $3(0.9)$ | $38(3.5)$ | $47(4.4)$ | $57(5.3)$ |
| $4(1.2)$ | $50(4.6)$ | $63(5.9)$ | $75(7.0)$ |
| $5(1.5)$ | $63(5.9)$ | $79(7.3)$ | $94(8.7)$ |
| $6(1.8)$ | $75(7.0)$ | $94(8.7)$ | $113(10.5)$ |
| $7(2.1)$ | $88(8.2)$ | $110(10.2)$ | $132(12.3)$ |
| $8(2.4)$ | $100(9.3)$ | $126(11.7)$ | $151(14.0)$ |
| $9(2.7)$ | $113(10.5)$ | $141(13.1)$ | $170(15.8)$ |
| $10(3.0)$ | $126(11.7)$ | $157(14.6)$ | $189(17.6)$ |
| $20(6.1)$ | $251(23.3)$ | $314(29.2)$ | $377(35.0)$ |
| $30(9.1)$ | $377(35.0)$ | $471(43.8)$ | $566(52.6)$ |
| $40(12.2)$ | $502(46.6)$ | $628(58.3)$ | $754(70.0)$ |
| $50(15.2)$ | $628(58.3)$ | $785(72.9)$ | $943(87.6)$ |

(a) Minimum Effective Depth of Pit $=10^{\prime}(3.0 \mathrm{~m})$.
(b) Sufficient area shall be provided for at least two bedrooms.
(c) Effective absorption surface for seepage pits includes sidewall areas only.

Table $\ddagger$ K-7 Soil Absorption Rate and Minimum Vertical Separation for Disposal Field Systems by Percolation Test Results

| Percolation Rate from Percolation Test minutes per in (minutes per cm.) | Shallow Disposal Field System ${ }^{2}$ (see footnote 2) Soil Absorption Rate gpd/ sq. ft. (lpd/sq. m.) | Deep Disposal Field System ${ }^{3-}$ (see footnote 3) Soil Absorption Rate gpd/sq. ft. (lpd/sq. m.) | Minimum Vertical Separation4-(see footnote 4) ft (m) |
| :---: | :---: | :---: | :---: |
| less than 1.00 | Note 1 | Note 1 | Note 1 |
| 1.00 to less than 3.00 $(1.18)$ | 1.20 (48.9) | 0.93 (37.9) | 40.0 (12.2) |
| 3.00 (1.18) | 1.10 (44.8) | 0.73 (29.7) | 10.0 (3.0) |
| 4.00 (1.57) | 1.00 (40.7) | 0.67 (27.3) | 10.0 (3.0) |
| 5.00 (1.97) | 0.90 (36.7) | 0.60 (24.4) | 10.0 (3.0) |
| 7.00 (2.76) | 0.75 (30.6) | 0.50 (20.4) | 10.0 (3.0) |
| 10.0 (3.94) | 0.63 (25.7) | 0.42 (17.1) | 10.0 (3.0) |
| 15.0 (5.91) | 0.50 (20.4) | 0.33 (13.4) | 5.0 (1.5) |
| 20.0 (7.87) | 0.44 (17.9) | 0.29 (11.8) | 5.0 (1.5) |
| 25.0 (9.84) | 0.40 (16.3) | 0.27 (11.0) | 5.0 (1.5) |
| 30.0 (11.81) | 0.36 (14.7) | 0.24 (9.8) | 5.0 (1.5) |
| 35.0 (13.78) | 0.33 (13.4) | 0.22 (9.0) | 5.0 (1.5) |
| 40.0 (15.75) | 0.31 (12.6) | 0.21 (8.6) | 5.0 (1.5) |
| 45.0 (17.72) | 0.29 (11.8) | 0.20 (8.1) | 5.0 (1.5) |

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| $50.0(19.68)$ | $0.28(11.4)$ | $0.19(7.7)$ | $5.0(1.5)$ |
| :--- | :--- | :--- | :--- |
| $55.0(21.65)$ | $0.27(11.0)$ | $0.18(7.3)$ | $5.0(1.5)$ |
| more than 55.0 to 60.0 <br> $(21.65$ to 23.62$)$ | $0.25(10.2)$ | $0.17(6.9)$ | $5.0(1.5)$ |
| more than 60.0 to 120 <br> $(23.62$ to 47.24$)$ | $0.20(8.1)$ | $0.13(5.3)$ | $5.0(1.5)$ |

Notes Footnotes:
${ }_{2}^{1} \frac{1}{2}$ Not permitted for septic tank effluent.
${ }^{z} \underline{2}$ Depth of bottom of disposal field below finished grade of soil is less than 5.00 feet $(1.5 \mathrm{~m})$.
${ }^{3} \frac{3}{3}$ Depth of bottom of disposal field below finished grade of soil is 5.00 feet $(1.5 \mathrm{~m})$ or greater.
${ }^{4} \underline{4}$ Minimum vertical distance required for achieving unsaturated flow is measured from the bottom of a constructed disposal field to the nearest restrictive soil condition including, but not limited to, the seasonal high water table capillary fringe, impermeable layer, rock, fractured rock, soils with greater than $50 \%$ rock fragments, and unacceptable soil.

## SEEPAGE PIT DESIGN




FIGURE $\mathbf{I - 2} \underline{K-2}$

## R4-48-128. Appendix J, Reclaimed Water Systems

A. Appendix J of the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code (1994 2000 Edition) is incorporated by reference. This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O. at 20001 Walmet Drive South, Walnut, CA, $91789-28255001$ E. Philadelphia St., Ontario, CA 91761 and are on file with the Arizona Uniform Plumbing Code Commission. and the Office of the Secretary of State.
B. Appendix J, incorporated by subsection (A) is modified as follows:

1. Appendix J2, paragraph 1, is modified to read: "Reclaimed water is water approved for specific uses after the water has been treated or processed by a wastewater treatment plant operated pursuant to statutes and rules of the Arizona Department of Environmental Quality. The level of treatment and quality of the reclaimed water for the uses specified in this appendix are established by the Arizona Department of Environmental Quality pursuant to Arizona Administrative Code R18-9-703.․ㅡ․
2. Appendix J2, paragraph 2, is deleted.

## R4-48-138. Installation-Standardfor Poly (VinylChloride)(PVC) NaturalGas-Yard Piping Reserved

## R4-48-143. Installation Standard for Łower Low Pressure Air Test for Building Sewers

This installation standard has no modifications.

## R4-48-147. Installation-Standard-for PEX AL-PEX and-PEAE-PE Trenchless Polyethylene (PE) Pipe for Sewer Laterals

The Uniform Plumbing Code incorporated in R4-48-102(A) is modified as follows:
Add a new installation standard for PEX-AL-PEX and PE-AL-PE:
ЄӨМРОSНЕ РЕХ АЦ-РЕХНӨТ АNЫ-РЕ-АЕ-РЕ-СӨЊР

## WАТЕR-ЭФТРझНТЮА SYSTEMS

This Standard shall govern the installation of composite piping in potable hot and cold water distribution systems within and under buildings and shall apply only to PEX-AL-PEX and PE-AL-PE piping meeting the requirements of ASTM F 1281 and ASTM F 1282. Installation, materials, and inspection should comply with the 1994 edition of the Uniform Plumbing Code as modified by this Chapter, published by the International Association of Plumbing and Mechanical Officials, and shall also eomply with this standard and manufacturer's installation recommendations.

NOTE: The following sections of the Uniform Plumbing Code shall apply to composite PEX-AL-PEX and PE-AL-PEpiping.
301.0 Materials Standards and Alternates
310.0 Workmanship
313.0 Protection of Piping, Materials, and Structures
314.0 Hangers and Supports
316.1.4 Flexible Compression Factory Fabrieated Joints
316.2.3 Plastic Pipe to Other Materials
606.0 Jeints and Connections
608.0 Water Pressture, Presstre Regulators, and Presstre Relief Valves
609.0 Installation, Unions, and Loeation
610.0 Size of Potable Water Piping

Note: The following elauses are the Installation Standard. Note that the Section numbers $301,313,314,316,606,608$, 609 , and 610 relate to the sections of the UPC but the specific clause numbers of this Installation Standard are not intended to mateh the UPC.

## Chapter 2 DEFINHTIONS

ASTM American Society for Testing and Materials
IAPMO International Association of Plumbing and Mechanical Officials
PEX-AL-PEXCrosslinked Polyethylene-Aluminum-Crosslinked Polyethylene
PE AL PEPolyethylene Aluminum Polyethylene
UPG Uniform Plumbing Code as published by IAPMO
301.0 Materials Standards and Alternates
301.1 Minimum Standards
301.1.1 Materials Materials shall comply with the following requirements:

Materials ASTM
Standard
Crosslinked Polyethylene-Altmintm-Crosslinked Polyethylene (PEX-AL-PEX) F 1281-01a
Polyethylene-Aluminum-Polyethylene (PE-AL-PE) F 1282-01a
Metal Insert Fittings for PEX AL PEX and PE AL PE composite pipe F1974-00a
301.1.1.1 Piping PEX AL PEX composite pipe shall comply with ASTM F 1281.

PE AL PE composite pipe shall comply with ASTM F 1282.
301.1.1.2 Fittings Fittings shall be metal insert type and shall comply with ASTM F 1974.

Manufacturers of fittings shall recommend assembly procedures. Fittings are limited to the following types:
(a) Insert fittings or compression type fittings,
(b) Special listed fittings of other types Connections to galvanized pipe or fittings shall be specifieally designed for that purpose.
301.1.2 Markings
301.1.2.1 Piping Composite piping shall be legibly marked at intervals of not more than $5^{\prime}(1.5 \mathrm{~m})$ with at least the following:
(a) Manufacturer's name or trademark;
(b) ASTMF 1281 (PEX AL PEX) or F 1282 (PE AL PE);
(e) Piping size;

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(d) Material type PEX AL PEX or PE AL PE;
(e) Pressure ratings at $125 \mathrm{psi}(862 \mathrm{kPa})$ at $180^{\circ} \mathrm{F}\left(82^{\circ} \mathrm{C}\right)(\text { PEX AL PEX })^{*}$ or, $200 \mathrm{psi}(1380 \mathrm{kPa})$ at $73^{\circ} \mathrm{F}\left(23^{\circ}\right.$ C) (PE-AL-PE)*
( $\ddagger$ ) Mark of an acceptable certification ageney;
(g) Manufacturer's date and material code

* The elevated temperature and presstre ratings for PEX AL PEX and PE AL PE in accordance with ASTM F1281 and ASTM F1282 are:

| PEX-AL-PEX (orange colored) | 200 psi at $73^{\circ} \mathrm{F}$ | 125 psi at $180^{\circ} \mathrm{F}$ |
| :--- | :--- | :--- |
| PE AL PE (blue colored) | 200 psi at $73^{\circ} \mathrm{F}$ | 160 psi at $140^{\circ} \mathrm{F}$ |

301.1.2.2 Fittings Fittings shall be marked with at least the following:
(a) Manufacturer's name or trademark or other acceptable markings; and
(b) Fittings shall be labeled with the mark of an aceeptable certification agency.
(c) If size permits, ASTM F1974.
301.1.2.3 Position of Markings When practical, markings shall be visible for inspection. Markings shall be visible prior to installation.
313.0 Protection of Piping, Materials, and Structures
313.1 Abrasion Piping passing through metallie studs, joists, or hollow masonry walls shall be protected from abrasion or sharp edges by elastomeric or plastic sleeves, grommets, conieal shaped puneh holes or other approved means.
313.2 Puncture Steel plate protection, minimum 18 gatge, shall be installed when the piping is within 1" (25 mm ) of the nailing surface.
313.3 Exposed Piping 313.3.1 General. Where exposed piping may be subjected to meehanieal damage it must be protected.
313.4 Freezing In areas where the system must be drained to protect the system from freezing, horizontal lines shall be graded to drain.
313.5 Storage. Piping shall be stored in a way to protect the system from mechanical damage (slitting, puncturing, ete.). Piping should be stored undereover to keep it clean and avoid long term expostre to sunlight. Consult piping manufacturer for recommended limits for outside storage.
313.6 Thermal Expansion
313.6.1 General The linear expansion rate for PEX-AL-PEX and PE-AL-PE is $1.56^{\prime \prime}(39.6 \mathrm{~mm})$ per $100^{\prime}(30 \mathrm{~m})$ of tube per $100 \times \mathrm{F}\left(55^{\circ} \mathrm{C}\right)$ change in temperature. No accommodation for thermal expansion is required.
313.6.2 Clearance-Bored holes and sleeves shall provide adequate clearance between the piping and strueture to allow for free longittudinal movement.
314.0 Hangers and Supports
314.1 Vertieal Piping Vertieal piping shall be supported at every floor. Piping shall have a mid-story guide.
314.2 Horizontal Piping Horizontal piping shall be supported according to the following Table $A$.

Fable-A. Suppert Spacing

| NominalPiameter | Spacing |
| :--- | :--- |
| $1 / 2 ", 3 / 4 "$, and $1^{\prime \prime}$ | $8^{\prime} 2^{\prime \prime}(2489 \mathrm{~mm})$ |

314.3 Hangers and Anchors Piping shall not be anchored rigidly to a support; but shall be seeured with hangers or straps that provide for a degree of movement and that prevent damage to the piping. Do not use hangers or straps with sharp or abrasive edges. Do not use hangers that pineh the piping.
314.4 Inspection and Testing
A. Inspection All piping shall be properly seated on to the fitting per the manufacturer instructions. Buekled, gouged or obviously damaged pipe shall not be used. Consult manufacturer recommendations for repair proeedures.
B. Festing Upen completion of a section or of the entire hot and cold water supply system it shall be tested and proved tight under a water pressure or air test not less than the working pressure under which it is to be used. The water used for tests shall be obtained from a potable sotrree. The system shall withstand the test without leaking for a period of not less than 15 minutes.
316.0 Jeints and Connections
316.1 Joints and Connections
316.1.1 Procedure-Piping should be cut with a pipe cutter designed specifically for composite pipe. Piping shall be eut square, i.e. perpendicular to the length. If other cutting methods are used care must be taken to remove any
excess material, flashing, or burrs.
316.1.2 Tools. The manufacturer's recommend fitting tool shall be used with the composite insert fitting systems. For specific procedtres, follow the mantufacturer's recommendations.
316.2 Special Jeints
316.2.1 Fittings Transitions for composite piping to metal piping or valves shall be made only with transition fittings intended for that purpose.
606.0 Joints and Connections
606.1 Joints Joints shall not be allowed in piping installed in or under a conerete slab resting on grade unless for repair within a building structure. All repair joints shall be properly protected with a heat shrink sleeve. All slab penetrations shall be sleeved.
608.0 Water Pressure, Pressure Regulators, and Pressure Relief Valves
608.1 PEX AL PEX Piping PEX AL PEX piping used for temperature and/or pressure relief valve drain lines shall be graded to the outlet end and shall be supported at a maximum of $8^{\prime} 2^{\prime \prime}(2489 \mathrm{~mm})$ interval horizontally. Vertical piping shall be supported at every floor. Vertical piping shall have a mid-story guide.
609.0 Installation, Unions, and Location
609.1 Bends Piping shall be installed by bending the composite pipe by hand to a minimum radius of five times the nominal pipe diameter. External bend supperts or sleeves are not required as the composite piping is rigid after bending.
609.2 Bamage Kinked, buekled, gouged, or other obvious damaged piping shall not be used.
609.3 Finish Nipples Finish nipples shall be connected to drop ear fittings to prevent rotation. Finish nipples shall not be PEX.
609.4 Hose Bibs. The piping directly connected to any hose bib shall be so anchored that the load on the hose bib will not strain the composite piping.
609.5 Heated Joints. An open flame shall not be applied to PEX-AL-PEX or PE-AL-PE piping when brazing, soldering, or welding joints.
609.6 Working Pressure and Temperature Long term working pressures for the PEX-AL-PEX shall not exceed a maximmm of $115 \mathrm{psi}(793 \mathrm{kPa})$ and the long term working temperature shall not exceed $210^{\circ} \mathrm{F}\left(99^{\circ} \mathrm{C}\right)$. Long term working pressures for the PE AL PE shall not exceed a maximum of 160 psi $(1103.2 \mathrm{kPa})$ and the long term Working temperature shall not exceed $140^{\circ} \mathrm{F}\left(60^{\circ} \mathrm{C}\right)$.
609.7 Exposure to Sunlight Only UV stabilized composite piping can be subjected to direct sunlight after instaltation and can be installed on the surface of the building. Kitec pipe contains an ultraviolet (UV) inhibitor to withstand limited exposure to UV light. Manufacturer recommends placing the unused portion of a Kitec coil back in the product's box rather than storing in the sunlight while not in use.
609.8 Water Heater Connections PEX-AL-PEX or PE-AL-PE piping shall not be installed within the first 18 " ( 457 mm ) of piping connected to a water heater.
609.10 Water Hammer Arrestors A composite hot water system will withstand repeated pressure surges, well in excess of its rated presstre. The Arizona Uniform Plumbing Code requires a means of attentating water hammer. Consequently water hammer arrestors may be advisable when solenoid valves or other quick closing devices are used in the system. In designing for such situations, it is advisable to consult the pipe or fittings manufacturer for recommended strge pressure limits. Water hammer and surge pressure caleulations are reviewed in Chapter 7, AWWA Mantal M-11.
610.1 Size of Potable Water Piping
610.1.1 Method Piping shall be sized in accordance with a Arizona Uniform Plumbing Code Section 610.0. When a Arizona Uniform Plumbing Code Appendix A is applieable, use Table B. Add equivalent lengths from Table $C$ when determining developed length. Maximum velocities through PEX-AL-PEX and PE-AL-PE copper alloy fittings shall be limited to $8^{\prime}$ per second $(\mathrm{fps})(2.4 \mathrm{mps})$ in cold water and 5 ' per secend ( fps ) $(1.52 \mathrm{mps})$ in hot water.

Table B. Head Loss ws. Flow Rate

| $\begin{gathered} \text { Flew-RateU.S. } \\ \text { GPM } \end{gathered}$ | 4/29 |  | 3/49 |  | 49 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Head Loss Psite.ft. | Velocity Ft./s | Head Loss Psi/e.ft. | Velocity F. $/ \mathbf{s}$ | Head Loss Psi/e.ft. | Velocity Ft./s |
| 0.1 | 0.02 | 0.2 | 0.002 | 0.07 | 0.001 | 0.04 |
| $\theta .2$ | $\theta .1$ | $\theta .4$ | 0.04 | $\theta .1$ | 0.002 | 0.08 |
| 0.3 | 0.2 | 0.6 | 0.02 | 0.2 | 0.005 | $\theta .1$ |
| 0.4 | 0.3 | 0.7 | 0.03 | 0.3 | 0.009 | 0.2 |
| 0.5 | 0.5 | $\theta .9$ | 0.04 | 0.3 | 0.04 | $\theta .2$ |

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| $\theta .6$ | \|0.6 | \| 1.4 | 0.05 | \|0.4 | 0.02 | 0.3 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0.7 | 0.9 | 1.3 | 0.07 | 0.5 | 0.02 | 0.3 |
| 0.8 | 1.4 | 1.5 | 0.09 | 0.5 | 0.03 | 0.3 |
| 0.9 | 1.4 | 1.7 | 0.4 | 0.6 | 0.04 | 0.4 |
| 1.0 | 1.6 | 1.8 | 0. 1 | 0.7 | 0.05 | 0.4 |
| 2.0 | 5.9 | 3.7 | 0.5 | 1.3 | 0.2 | 0.9 |
| 3.0 | 12.5 | 5.5 | 1.0 | 2.0 | 0.4 | 1.3 |
| 4.0 | 21.3 | 7.3 | 1.8 | 2.6 | 0.6 | 1.7 |
| 5.0 |  |  | 2.7 | 3.3 | 0.9 | 2.1 |
| 6.0 |  |  | 3.8 | 4.0 | 1.3 | 2.5 |
| 7.0 |  |  | 5.0 | 4.6 | 1.7 | 3.0 |
| 8.0 |  |  | 6.4 | 5.3 | 2.2 | 3.4 |
| 9.0 |  |  | 8.0 | 5.9 | 2.7 | 3.8 |
| 10.0 |  |  | 9.7 | 6.6 | 3.3 | 4.2 |
| 11.0 |  |  | 11.6 | 7.2 | 3.9 | 4.6 |
| 12.0 |  |  | 13.6 | 7.9 | 4.6 | 5.0 |
| 13.0 |  |  |  |  | 5.3 | 5.5 |
| 14.0 |  |  |  |  | 6.1 | 5.9 |
| 15.0 |  |  |  |  | 6.9 | 6.3 |
| 16.0 |  |  |  |  | 7.8 | 6.3 |
| 17.0 |  |  |  |  | 8.7 | 6.7 |
| 18.0 |  |  |  |  | 9.7 | 7.4 |
| 19.0 |  |  |  |  | 10.7 | 7.6 |
| 20.0 |  |  |  |  | 11.8 | 8.0 |

Fable C. Developed Length

| Sizes, Imehes | Type of Fittings | Equivalent Lenghth-of Рipe (Feet) |
| :---: | :---: | :---: |
| 1/2 | Couplings <br> Adapters <br> Elbows <br> Tees (Branch Flow) <br> Tees (On the Run) | $\begin{aligned} & \hline z \\ & z \\ & 7.5 \\ & 8 \\ & 2.5 \end{aligned}$ |
| 3/4 | Couplings <br> Adapters <br> Elbows <br> Fees (Branch Flow) <br> Tees (On the Run) | $\begin{aligned} & \hline z \\ & z \\ & 8.5 \\ & 10.5 \\ & 2.5 \end{aligned}$ |
| 4 | Couplings <br> Adapters <br> Elbows <br> Tees (Branch Flow) <br> Tees (On the Run) | $\begin{aligned} & \hline z \\ & z \\ & 9 \\ & 14 \\ & 2.5 \end{aligned}$ |

Incorporate by reference the International Association of Plumbing and Mechanical Officials' (I.A.P.M.O.) Uniform Plumbing Code ( 2000 Edition). This incorporation by reference does not include any later amendments or editions. Copies of the incorporated material are available from I.A.P.M.O., 5001 E. Philadelphia St., Ontario, CA 91761 and are on file with Arizona Uniform Plumbing Code Commission.

## NOTICE OF PROPOSED RULEMAKING

Editor's Note: The following Notice of Proposed Rulemaking is reprinted. It was originally published at 10 A.A.R. 3257, August 20, 2004. In the original printing, R18-5-104(A) and R18-5-109(A) and (B) were inadvertently shown as stricken text. In this reprint, the errors have been corrected and the language is not indicated as stricken text.

## TITLE 18. ENVIRONMENTAL QUALITY

## CHAPTER 5. DEPARTMENT OF ENVIRONMENTAL QUALITY ENVIRONMENTAL REVIEWS AND CERTIFICATION

PREAMBLE

1. Sections Affected

Article 1
R18-5-101
R18-5-104
R18-5-107
R18-5-109
R18-5-115

## Rulemaking Action

Amend
Amend
Amend
Amend
Amend
Amend
2. The statutory authority for the rulemaking, including both the authorizing statute (general) and the statute the rules are implementing (specific):

Authorizing statutes: A.R.S. §§ 49-104, 49-202, 49-203, 49-351, 49-352, 49-353 and 49-361
Implementing statute: A.R.S. §§ 49-352, 49-361
3. A list of all previous notices appearing in the Register addressing the proposed rule:

Notice of Rulemaking Docket Opening: 10 A.A.R. 1319, April 2, 2004
4. The name and address of agency personnel with whom persons may communicate regarding the rulemaking:

Name: Jon Fiegen or Sean McCabe
Address: Arizona Department of Environmental Quality
1110 W. Washington St. (MC 5415B-2)
Phoenix, AZ 85007
Telephone: Jon Fiegen (602) 771-4596
Sean McCabe: (602) 771-4600
(Toll-free number in Arizona: (800) 234-5677)
Fax: (602) 771-4634
E-mail: fiegen.jon@azdeq.gov
mccabe.sean@azdeq.gov
5. An explanation of the rule, including the agency's reasons for initiating the rule:
A. Background for Proposed Rules

The primary rationale for this rulemaking is to improve the operator certification program administered by the Arizona Department of Environmental Quality (ADEQ), which certifies operators of drinking water and wastewater facilities. The requirements relating to the certification of public water system operators come primarily from the federal Safe Drinking Water Act (SDWA), which seeks to ensure that drinking water supplied to consumers by public water systems is safe to drink, that consumers are confident that their water is safe to drink and that certified operators operate public water systems. The Environmental Protection Agency (EPA) promulgated final guidelines for the certification and recertification of water system operators in February 1999. 64 FR 5916 through 5921, February 5, 1999.

Currently, ADEQ is able to revoke the certificate of an operator of a wastewater or water facility for noncompliance pursuant to A.A.C. R18-5-109. A proposed amendment to these rules will allow ADEQ also to place an operator on probation or suspension for noncompliance pursuant to A.A.C. R18-5-109, among other changes.
B. Section-by-section Explanation of the Rules

R18-5-101 sets forth definitions for this Article.
R18-5-104 sets forth general requirements for the operation of water and wastewater facilities and specific requirements for the owners and operators of water and wastewater facilities.

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R18-5-107 sets forth the requirements for certificate renewal for operators of water and wastewater facilities.
R18-5-109 sets forth the enforcement mechanisms the Department may employ with operators of water and wastewater facilities.R18-5-115 sets forth the criteria for classifying the grades of water treatment and water distribution facilities.
6. A reference to any study relevant to the rule that the agency reviewed and either proposes to rely on in its evaluation of or justification for the rule or proposes not to rely on in its evaluation of or justification for the rule, where the public may obtain or review each study, all data underlying each study, and any analysis of each study and other supporting material:

None
7. A showing of good cause why the rule is necessary to promote a statewide interest if the rule will diminish a previous grant of authority of a political subdivision of this state:

The proposed amendments do not diminish a previous grant of authority of a political subdivision of this state.
8. The preliminary summary of the economic, small business, and consumer impact:
A. Identification of Rule

Title 18, Chapter 5, Article 1, "Classification of Treatment Plants and Certification of Operators."

## B. Background and Summary

Arizona has a Safe Drinking Water program approved by the EPA that governs drinking water, public water systems and operators of public water systems. ADEQ also regulates wastewater facilities and wastewater facility operators. Currently, ADEQ may only revoke the certificate of an operator for noncompliance. Possessing only operator certificate revocation creates a program with a "one-size-fits-all" enforcement mechanism. The expanded enforcement mechanism proposed by this rulemaking will provide ADEQ with greater flexibility and an increased ability to take corrective actions with water and wastewater facility operators that are appropriate to the violations committed.
Additionally, EPA calls upon states with an approved drinking water program to have the ability to suspend a public water system operator's certificate for noncompliance, or take other appropriate enforcement measures. Since EPA calls upon states with an approved drinking water program to have this ability, the adoption of suspension and probation as additional enforcement measures will assist ADEQ in maintaining its approval from EPA to implement the Safe Drinking Water Act in Arizona. The adoption of suspension and probation as additional enforcement measures to address noncompliance by operators of wastewater facilities will also assist ADEQ in protecting the public health, safety and the environment.
This proposed rulemaking makes changes to five of the sixteen sections comprising Article 1. ADEQ believes that the results of these changes will represent cost-saving benefits in terms of improved protection to public health and safety. For example, the marginal costs to owners of water and wastewater facilities from replacing the services of an operator placed on suspension are expected to be less than the potential value of marginal benefits to consumers and general public.
The current rule allows ADEQ to deny or revoke an operator certificate. If, however, these proposed rule changes are implemented, ADEQ will be able to suspend a certificate or place an operator on probation for violating provisions of these rules (see R18-5-109). Potentially, these additional enforcement mechanisms can provide better protection of the public health and the environment. This is vital to Arizona's program since the primary purpose of the Safe Drinking Water Act is to ensure that drinking water supplied to consumers by public water systems is safe to drink. ADEQ must also have this same enforcement authority for wastewater facility operators for the protection of public health and the environment.

Although potential costs and benefits are described in part " D " below, ADEQ anticipates the incremental cost to implement this rule probably will be minimal. ADEQ solicits additional information on entities impacted and potential costs and benefits. Information and data received during the oral proceeding will be evaluated and included, as appropriate, in the final rulemaking.

## C. Entities Directly Impacted

Entities that could be directly affected bear costs, or directly benefit from this rule include: owners of water facilities and wastewater facilities, operators, ADEQ, and the public. Water and wastewater facilities include private and public owners. The federal government, state agencies, various political subdivisions of the state and private entities own and operate these facilities and could be impacted by this rulemaking. The public includes consumers and citizens at large.
ADEQ does not anticipate an impact to other state agencies or to the state General Fund.

## D. Potential Costs and Benefits

Owners of Facilities. The potential for increased operating costs for owners of water facilities and wastewater facilities may arise for facilities that would find it necessary to replace the services of an operator who has been suspended. A facility faced with the necessity of finding replacement services for a suspended operator may encounter increased
transaction costs to find a replacement operator. In addition, the cost for a certified operator may or may not be equivalent to prior service costs. However, if transaction costs are minimal and replacement service costs are relatively equivalent to prior costs, the impact should be "minimal." In cases where transaction costs become more expensive and replacement service costs exceed what the facilities were previously paying, the relative impact should be greater.
Even though the potential does exist for higher costs, ADEQ anticipates that significantly higher costs would be the exception rather than the anticipated scenario. Furthermore, relatively larger facilities may not experience any impact in the event of an operator suspension because of multiple operators already employed at those facilities. Finally, the threat of suspension or probation may actually work as an incentive for existing and new operators to perform their job tasks and to operate the facilities in compliance with the law.
Certified operators. Certified operators are persons who hold a current certificate issued by ADEQ. Earning professional development hours (PDHs) for certificate renewals is an existing requirement. Although the rule currently requires an operator to provide PDH records to ADEQ upon request, the requirement to maintain the documentation for at least five years would be a new requirement under these proposed rules. ADEQ views the impact of this new requirement to be minimal. Although the rule provision that authorizes ADEQ to suspend an operator's certificate or place an operator on probation, represents a potential increased risk to these operators, the outcome, including improved operator performance, may be viewed as a positive and beneficial one.
ADEQ. In addition to the minimal rule development costs incidental to the rulemaking process, ADEQ does not anticipate the need for additional employees, equipment, or other explicit and implicit costs to accrue as a result of implementing this rulemaking. Even though the ability to suspend an operator's certificate or place an operator on probation would fall under a category of increased enforcement costs, ADEQ does not anticipate a significant increase in enforcement costs.

ADEQ expects that the implementation of these proposed rule changes will strengthen the program that certifies and regulates the operators of water and wastewater facilities. Drinking water that is safe to drink is of critical importance to everyone. The proper operation of public water systems that deliver water to consumers is a vital step in maintaining safe drinking water. Similarly, it is important to public health and safety and the environment that wastewater is adequately treated through the proper operation of wastewater facilities. The adoption of suspension and probation as additional enforcement mechanisms for wastewater and water facility operators will improve ADEQ's oversight over these programs and will assist ADEQ in maintaining its approval from EPA to implement the Safe Drinking Water Act in the state.

Consumers and general public. Initially, this rulemaking is not expected to generate costs to consumers or to the general public. However, the potential does exist for owners/operators to pass on increased compliance costs to consumers. ADEQ expects probable benefits to outweigh probable costs because of the potential for increased public protection. In some cases, the potential exists for cost-avoidance benefits to facilities due to improving the operation of systems and mitigating the potential for facilities being out of compliance. Thus, this rulemaking can be viewed as a direct link to improving the well-being of consumers and the general public.

## E. Rule Impact Reduction on Small Businesses

State law requires agencies to reduce the impact of a rule on small businesses by using certain methods when they are legal and feasible in meeting the statutory objectives of the rulemaking. ADEQ considered each of the methods prescribed in A.R.S. §§ 41-1035 and 41-1055(B)(5)(c) for reducing the impact on small businesses. Methods that may be used include the following: (1) Exempt them from any or all rule requirements, (2) Establish performance standards which would replace any design or operational standards, or (3) Institute reduced compliance or reporting requirements. An agency may accomplish the 3rd method by establishing less stringent requirements, consolidating or simplifying them, or setting less stringent schedules or deadlines.
After assessing the various methods for reducing the impact on small businesses, ADEQ has been unable to implement any of the suggested methods. Owners and operators of these facilities must meet these rules to maintain an adequate and appropriate Safe Drinking Water program and wastewater program in Arizona.
9. The name and address of agency personnel with whom persons may communicate regarding the accuracy of the economic, small business, and consumer impact statement:

A person may submit written comments to the individuals listed in item \#4.

## 10. The time, place, and nature of the proceedings for the making, amendment, or repeal of the rule or, if no proceed-

 ing is scheduled, where, when, and how persons may request an oral proceeding on the proposed rule:Date: October 5, 2004
Time: 1:30 p.m.
Location: Department of Environmental Quality 1110 W. Washington, Room 250 Phoenix, AZ 85007
(Please call 602-771-4795 for special accommodations pursuant to the Americans with Disabilities Act.)

The public record on the proposed rulemaking will close at 5:00 p.m., October 7, 2004. Oral comments may be made at the proceeding referenced above; written comments must be received by the close of record date. Address written comments to one of the individuals identified in item \#4.

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11. Any other matters prescribed by statute that are applicable to the specific agency or to any specific rule or class of rules: None
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12. Incorporations by reference and their location in the rules:

None

## 13. The full text of the rules follows:

## NOTICE OF PROPOSED RULEMAKING

## TITLE 18. ENVIRONMENTAL QUALITY

## CHAPTER 5. DEPARTMENT OF ENVIRONMENTAL QUALITY ENVIRONMENTAL REVIEWS AND CERTIFICATION

## ARTICLE 1. CLASSIFICATION OF TREATMENT PLANTS WATER AND WASTEWATER FACILITIES AND CERTIFICATION OF OPERATORS

## Section

R18-5-101. Definitions
R18-5-104. General Requirements
R18-5-107. Certificate Renewal
R18-5-109. Denial, Suspension, Probation and Revocation
R18-5-115. Grades of Water Treatment Plants and Distribution Systems

## ARTICLE 1. CLASSIFICATION OF TREATMENT PLANTS WATER AND WASTEWATER FACILITIES AND CERTIFICATION OF OPERATORS

## R18-5-101. Definitions

The terms in this Article have the following meanings:
"Certified operator" or "operator" means an individual who holds a current certificate issued by the Department in the field of water or wastewater treatment, water distribution, or wastewater collection, and is respensible for the daily onsite operation or the remote operation from a central location of all or a part of a facility.
"Collection system" means a pipeline or conduit, a pumping station, a force main, or any other device or appurtenance used to collect and conduct wastewater to a central point for treatment and disposal.
"Department" means the Department of Environmental Quality or its designated representative.
"Director" means the Director of the Department of Environmental Quality or the Director's designated representative.
"Direct responsible charge" means day-to-day decision making responsibility for a facility or a major portion of a facility.
"Distribution system" means a pipeline, appurtenance, or device of a public water system that conducts water from a water source or treatment plant to consumers for domestic or potable use.
"Facility" means a water treatment plant, wastewater treatment plant, distribution system, or collection system.
"Industrial waste" means the liquid, gaseous, or solid waste produced at an industrial operation.
"Onsite operator" means an operator who visits a facility at least daily to ensure that it is operating properly.
"Onsite representative" means a person located at a facility who monitors the daily operation at the facility and maintains contact with the remote operator regarding the facility.
"Operator" has the same meaning as certified operator, defined in this Section.
"PDH" means professional development hour, defined in this Section.
"Population equivalent" means the population that would contribute an equal amount of biochemical oxygen demand (BOD) computed on the basis of 0.17 pounds of five-day, 20-degree centigrade BOD per capita per day.
"Professional development hour" or "PDH" means one hour of participation in an organized educational activity related to engineering, biological or chemical sciences, a closely related technical or scientific discipline, or operations management.
"Public water system" has the same meaning prescribed in A.R.S. § 49-352.
"Qualifying discipline" means engineering, biology, chemistry, or a closely related technical or scientific discipline.
"Qualifying experience" means experience, skill, or knowledge obtained through prior employment that is applicable to the technical or operational control of all or part of a facility.
"Remote operator" means an operator who is not an onsite operator.
"Validated examination" means an examination that is approved by the Department after being reviewed to ensure that the examination is based on the class and grade of a system or facility.
"Wastewater" means sewage, industrial waste, and all other waterborne waste that may pollute any lands or waters of the state.
"Wastewater treatment plant" means a process, device, or structure used to treat or stabilize wastewater or industrial waste and dispose of the effluent.
"Water treatment plant" means a process, device, or structure used to improve the physical, chemical, or biological quality of the water in a public water system.

## R18-5-104. General Requirements

A. A facility owner shall ensure that at all times:

1. A facility has an operator in direct responsible charge who is certified for the class of the facility and at or above the grade of the facility.
4.2. Only a certified operator can make a decision An operator makes all decisions about process control or system integrity regarding water quality or water quantity that affects public health; however, an administrator who is not a certified operator can make a planning decision regarding water quality or water quantity as long as the decision is not a direct operational process control or system integrity decision that affects public health.
2. The operator in direct responsible charge of the facility is certified for the class of facility at which the operator works, and at or above the grade of the facility for which the operator works;
3. An operator who is in direct responsible charge of more than one facility is certified for the class of each facility and at or above the grade of the facility with the highest grade;
4. An operator who replaces the operator in direct responsible charge does not begin operation of the facility before being certified for the applicable class and grade of the facility;
5. In the absence of the operator in direct responsible charge, the operator in charge of the facility is certified for the applicable class of facility and at a grade no lower than one grade below the grade of the facility; and
6. The names of all current operators are on file with the Department.
B. If the owner of a facility replaces an operator in direct responsible charge with another operator, the facility owner shall notify the Department in writing within ten days of the replacement.
C. The operator shall notify the Department in writing within ten days of the date the operator either ceases operation of a facility or commences operation of another facility.
D. A facility owner shall ensure that an operator holding eertification in a particular class and grade only operates a facility of the same elass and the same or lower grade for which the operator is certified. An operator shall operate each facility in compliance with applicable state and federal law.
E. A facility owner shall ensure that a Grade 3 or Grade 4 facility has an onsite operator.
F. An operator holding certification in a particular class and grade may operate one or more Grade 1 or Grade 2 facilities as a remote operator if the facility owner ensures that the following requirements are met:
7. The remote operator is certified for the class of each facility and at or above the elass and grade of each facility operated by the remote operator.
8. There is an onsite representative on the premises of each Grade 1 or Grade 2 facility, except for a Grade 1 water distribution system that serves fewer than 100 people, which is not required to have an onsite representative if the conditions of $(\mathrm{E})(8)(\mathrm{F})(8)$ are met. The onsite representative is not required to be an operator if the facility has a remote operator who is certified at or above the grade of the facility.
9. The remote operator instructs, supervises and provides written instructions to the onsite representative in the proper operation and maintenance of each facility, providing written instructions, and ensuring ensures that adequate records are kept.
10. The remote operator provides the onsite representative with a telephone number at which the remote operator can be reached at all times. If the remote operator is not available for any reason, the remote operator shall provide the onsite representative with the name and telephone number of a qualified substitute operator who will be available while the remote operator is not available.
11. The remote operator resides no more than 200 miles by ground travel from any facility that the remote operator serves.
12. The remote operator operates each facility in compliance with applicable state and federal laws.
13. The remote operator inspects a facility as often as necessary to assure ensure proper operation and maintenance, but in no case less than:
a. Monthly for a Grade 1 or Grade 2 water treatment plant or distribution system that produces and distributes groundwater;
b. Monthly for a Grade 1 wastewater treatment plant;
c. Twice a month for a collection system that serves fewer than 2,500 people; and
d. Weekly for a Grade 2 wastewater treatment plant or collection system that serves fewer than 1,000 people.

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8. For a Grade 1 water distribution system that does not have an onsite representative and serves fewer than 100 people, the following conditions are met:
a. The name and telephone number at which the remote operator can be reached is posted at the facility, enclosed with water bills, or otherwise made readily available to water users. If the remote operator is not available for any reason, the remote operator shall post at the facility the name and telephone number of a substitute operator of the applicable facility class and grade who will be available while the remote operator is not available.
b. The remote operator or substitute operator resides no more than 200 miles by ground travel from the facility; and
c. The remote operator inspects the facility weekly.

## R18-5-107. Certificate Renewal

A. If the Department renews a certificate, the certificate is renewed for three years, unless the operator requests a shorter renewal period in writing.
B. To renew a certificate, an operator shall maintain documentation and provide it to the Department upon request to verify completion of at least 30 PDHs accumulated during a certification period. The operator shall provide documentation of PDHs that is in a format acceptable to the Department. At least 10 of the PDHs shall directly relate to the specific job functions of the operator. If an operator holds multiple certificates, the required PDHs may be applied to all certificates if the PDHs are acquired within that certification period. The operator's supervisor or the entity that provides the education or training shall verify completion of each PDH in writing. An operator shall maintain documentation of completion of PDHs for a minimum of five years.
C. As an alternative to the requirements of subsection (B), an operator may renew a certificate by taking and passing an examination for the applicable class and grade.

## R18-5-109. Denial, Suspension, Probation and Revocation

A. The If the Department decides to deny, suspend, or revoke a certificate, or to place an operator on probation, the Department shall act in accordance with shall act under A.R.S. Title 41, Chapter 6, Article 10 and 18 A.A.C. 1, Article 2 to deny or revoke a certifieate.
B. If it is In determining whether to revoke or suspend a certificate, or to place an operator on probation, or in determining the length of suspension or probation, the Department shall consider whether the operator:

1. Operates a facility in a manner that violates federal or state law;
2. Negligently operates a facility or negligently supervises the operation of the a facility;
3. Fails to comply with a Department erders order or eensent decrees order of a court;
4. Obtains a certificate by fraud, deceit, or misrepresentation;
5. Engages in fraud, deceit, or misrepresentation in the operation or supervision of a facility;
5.6. Knowingly or negligently prepares a false or fraudulent report or record regarding the operation or management supervision of the a facility; or
6.7. Endangers the public health, safety, or welfare;
6. Fails to comply with the terms or conditions of probation or suspension; or
7. Fails to cooperate with an investigation by the Department.
C. The Department shall deny certification to an applicant who does not meet the requirements of R18-5-105 or R18-5-110, or who is ineligible for certification pursuant to a Department order or order of a court.
E.D. In order to be recertified, a person whose certificate is revoked shall reapply and be reexamined as a new applicant. A person whose certificate is revoked is not eligible for admission to a certification examination for 12 months from the effective date of the revocation.
E. The Department may place an operator on probation to address deficiencies in operator performance. The terms of probation may include completion of additional PDHs, increased reporting of operator activity, limitations on activities the operator may perform, or other conditions.
F. During the period of suspension or revocation, a person whose certificate is suspended or revoked shall not operate a facility of any class or grade.
G. A person whose certificate is suspended or revoked shall immediately notify the owner of a facility where the operator is employed of the suspension or revocation.

## R18-5-115. Grades of Water Treatment Plants and Distribution Systems

A. Grading of water treatment plants. This subsection does not apply to a facility that distributes water but does not treat water or to a facility that distributes water and disinfects by chlorine gas or hypochlorite only. The Department shall grade a water treatment plant according to the sum of the points it assigns for each plant characteristic.

1. No Change
2. No Change
B. Grading of water distribution systems. The Department shall grade a distribution system according to the sum of the points it assigns for each system characteristic.
3. The Department shall assign points for the purpose of grading a distribution system as follows:

| System Characteristics | Points |
| :--- | :--- |
| Population | 1 per 5,000 |
| Maximum Design Capacity | 1 per Millions of Gal- <br> lons per Day up to 10 |
| Pressure Zones | 5 |
| Booster Stations | 5 |
| Storage Tanks | 3 |
| Blending | 5 |
| Fire Protection Systems/Testable <br> Backflow Prevention Assemblies | 5 |
| Cathodic Protection | 3 |
| Control System Technologies | 2 |
| Chlorine Gas | 6 |
| Hypochlorite Liquid | 2 |
| Hypochlorite Solid | 2 |
| Chloramine | 9 |
| Chlorine Dioxide | 9 |

2. No Change
a. No Change
b. No Change
c. No Change
d. No Change
3. No Change
